

# Conclusions and Prospects

The slope stability under seismic effects is a both ancient and young subject. Because of its high complexity, the development till now is still immature. The special geographical and geological conditions and seismic configuration make it especially pertinent in China. Currently, the infrastructure construction such as highway, railway, and water conservancy is in rapid developing period, which draws higher demand on seismic design of slope engineering. Therefore, the slope stability under seismic effects is a crucial scientific problem urgently needs to be solved.

## Part One: The Major Work and Conclusion of the Dissertation

Surrounding the national demands on infrastructure construction and reconstruction of Wenchuan seismic areas, and based on the investigated data of Wenchuan seismic slope damage, the research results of seismic array monitoring, the large-scale shaking table test and the numerical analysis, and technical measures like theoretical analysis, we explored the dynamic features of rock slope. On this basis, we take the most seriously hit bedrock and overburden slope in the earthquake as study object. This dissertation systematically studied on the scientific issues such as the rock slope dynamic response rules under seismic effects, the formation mechanism of landslide, and stability evaluation method as well as landslide hazard evaluation, which covered problems of slope stability under seismic effects on micro- and macro-aspects, and achieved some results and conclusions in the following aspects.

### On Basic Dynamic Response of Rock Slope

- (1) Based on the seismic array monitoring results of Zigong topography in “5.12 Wenchuan earthquake,” we conducted research on the seismic dynamic characteristics of slope topography from the aspects of amplitude response,

Fourier spectrum, and response spectrum and achieved the following results: The component PGA of each direction on slope topography increases in nonlinear trend along elevation, the acceleration elevation amplification effects on EW direction were stronger than those on NS direction and those on UP direction were the weakest; for the horizontal acceleration, with the increase of elevation, the frequency components of Fourier spectrum around the slope natural frequency had amplification effects and showed the changing process from single peak to double peak, while the Fourier spectrum of vertical acceleration basically stayed unchanged; with the increase of elevation, the dynamic amplification coefficient of acceleration response spectrum increased, so did amplification effects, and the dynamic amplification coefficient of horizontal acceleration response spectrum was more than that of vertical acceleration response spectrum (Chap. 2).

- (2) This chapter based on the large-scale shaking model test studied on the influence of bevel angle, seismic intensity, and seismic wave input type on rock slope dynamic characteristics, and its results showed: For mountains containing four slope surfaces, if we analyze with traditional method, i.e., using  $X$ ,  $Y$ , and  $Z$  three direction coordinate system to analyze PGA elevation amplification effects, the results may be unilateral, so it suggested to use the three-dimensional local coordinate system. i.e., the free face direction, slope strike direction, and vertical direction of slopes; with the increase of bevel angles, the PGA elevation amplification effects in vertical direction will rise gradually, while at the same time, there exists a sudden increase turning point at bevel angles of  $45^\circ$  and a leveling off turning point at  $50^\circ$ . However, the PGA elevation amplification effects in slope strikes stay unchanged with the increase of bevel angles, and its steps are comparatively gentle; with the increase of seismic ground motion PGA, the peak acceleration elevation amplification effect decreases gradually, slope strike direction and vertical direction reduce gradually and show characteristics of magnitude saturation; the changes of Fourier spectrum frequency components of acceleration in free face direction ( $L$ ), slope strike ( $M$ ), and vertical direction ( $N$ ) along elevation have certain regularity, i.e., with the increase of elevation, the slope soil has significant amplification effects for frequency components around its natural frequency range  $f$  and has filter effects for other frequency ranges; the changes of Fourier spectrum frequency components of acceleration in free face direction ( $L$ ), slope strike ( $M$ ), and vertical direction ( $N$ ) along elevation are basically identical, and the correspondent response spectrum amplitude of predominant period has certain amplification effects along elevation, while for other period  $T$ , especially long period parts (low-frequency parts), there exists certain reduction effects. In addition, it has obvious peak phenomena for free face direction, while for slope strike direction and vertical direction, the peak phenomena are less obvious (Chap. 3).
- (3) This chapter adopted both shaking table test and numerical analysis, and studied the influence of factors such as slope type, slope top local topography on the rock slope dynamic characteristics under seismic effects from the

perspective of time domain and joint time–frequency domain, and the results show: The acceleration Fourier spectrum along elevation of single- and double-sided high and steep slope surfaces all shows the changing rules of low-frequency components amplification, and the changing rules are influenced by slope topography and bevel angles. No matter for horizontal acceleration or vertical acceleration, the acceleration response spectrum along elevation of single-sided slope surfaces all shows single peak value, while that of double-sided slope surfaces followed the process of converting from single peak value to double peak value; no matter for single-sided high and steep slope or for double-sided high and steep slope, the PGA of different positions along elevation all amplifies to different extent, and it shows as follows: The amplification effects of double-sided high and steep slope on acceleration > the amplification effects of single-sided high and steep slope on acceleration; PGA amplification effects on slope surface > PGA amplification effects inside slope mass; the horizontal acceleration coefficient > the vertical acceleration coefficient, and acceleration coefficient on steep slope topography > acceleration coefficient on gentle slope topography; the local topography of slope top has significant influence on the intensity of seismic response, and the detailed rules are as follows: three peak slope top > double peak slope top > single peak slope top > flattop; and the high-frequency components of all of them gradually strengthen and the second step frequency becomes gradually obvious. At the same time, the characteristic periods of acceleration response spectrum of flattop, single, double, and three peak slopes are gradually transferred from long period to short period, and the double peak value of response spectrum gradually becomes obvious (Chap. 4).

## **On Theoretical Analysis of Rock Slope Acceleration Elevation Amplification Effects**

- (4) The double-sided rock slope acceleration elevation amplification effects time–frequency method is derived, based on elastic wave theory and horizontal slice method, through building force equilibrium differential equation of micro-unit, using Hilbert–Huang Transform and boundary conditions, and applying modal analysis method and normal mode theory. The rationality of this method is verified through shaking model test and numerical analysis. This method could not only take good consideration of the influence of three essential elements (PGA, frequency, and duration) on acceleration elevation amplification, but also explain the “whiplash effects” of slopes in the perspective of theoretical calculation. With the increase of seismic intensity, the shear displacement at slope top gradually increases, and the acceleration amplification coefficient gradually decreases; the shear displacement of slope mass is in nonlinear distribution along elevation and suddenly increases in multiple times; with the

increase of input wave frequency, the maximum shear displacement and acceleration amplification coefficient at slope top are distributed in saddle shape and reach the maximum value when input frequency value equals the maximum value of the natural frequency of slope mass; with the increase of bevel angles, the maximum shear displacement and acceleration amplification coefficient all gradually increase, and distribute in “step-form,” i.e., in the ranges of  $30^{\circ}$ – $45^{\circ}$  and  $50^{\circ}$ – $60^{\circ}$ , the amplification coefficient all increases slightly, while in the range of  $45^{\circ}$ – $50^{\circ}$ , it increases suddenly. Through comparative analysis of high and steep rock slope time–frequency analysis method, with the calculation results of acceleration elevation amplification effects analysis method regulated in Code for Seismic Design of Buildings GB50011-2010, Specifications for Seismic Design of Hydraulic Structures DL/5073-2000, and Design Specification for Slope of Hydropower and Water Conservancy Project SL/386-2007, it is suggested that the calculation of rock slope acceleration elevation amplification coefficient in Seismic Code, Hydraulic Seismic Specifications, and Slope Design Specifications should take consideration of the influence of seismic intensity, so as to improve the rationality of structural seismic design (Chap. 5).

## **On Slope Deformation Characteristics and Formation Mechanism of Bedrock and Overburden Slopes**

- (5) This chapter focused on the two major landslide hazard spots on the left side of G213, built the numerical analysis model based on the shaking table test model, and used the new numerical calculation software—GDEM to simulate the whole process of deposit body landslides of single- and double-sided bedrock and overburden slopes from deformation to failure slip under intense earthquake, conducting comprehensive and in-depth research of slope deformation features and formation mechanism of bedrock and overburden slopes from the perspective of time domain and joint time–frequency domain. (1) The results show: No matter for single- or double-sided bedrock and overburden slopes, the landslides processes are basically the same, i.e., under effects of gravity and seismic power, tensile stress concentration firstly occurs at sliding mass top, causing deformation of sliding mass along sliding mass structural surface trailing edge, which causes tensile and shear failure points at this position. Then, with continuous seismic power, the shear failure points on sliding mass structural surface gradually develop to lock-fixed section of sliding mass leading edge with increasing amount of tensile failure points on sliding mass surface, which finally causes progressive damage of lock-fixed section, crack surface connecting to sliding zone and sliding mass slipping from shear crack into landslide; the inconsistency of motion between sliding bed and sliding mass, difference of seismic energy distribution and dissipation,

and decrease of sliding mass structural surface strength with increase of seismic intensity are three major controlling factors inducing landslides. When seismic ground motion acceleration is small, the instant frequency of horizontal acceleration inside the sliding bed and sliding mass stabilizes in a certain range with the former slightly higher than the latter. At the same time, the energy transmission coefficient on the sliding mass structural surface and the controlling frequency range of seismic energy inside the sliding mass all stabilizes in a certain range; with the increase of seismic intensity, the instant frequency of acceleration inside the sliding mass gradually decreases, and the energy transmission coefficient of sliding mass structural surface gradually decreases, with both finally approaching stability. Simultaneously, the controlling frequency range of seismic motion energy inside the sliding mass transfers from high-frequency range to low-frequency range (Chap. 6).

### **On Seismic Stability Time–Frequency Analysis Method of Bedrock and Overburden Slope**

- (6) Based on elastic wave theory and general geological analysis model, this chapter derived seismic stability time–frequency analysis method of bedrock and overburden slope, and verified its rationality through shaking table test and numerical analysis results. This method could not only consider the influence of three essential elements (PGA, frequency, and duration) of SV wave on slope stability, but can also predict the safety of rock slopes, the happening time, and scale of landslides. At the same time, it can provide reference to supporting structure seismic time–frequency design in high intensive seismic regions; at the same time, the elasticity modulus of incident angle and cover layer has significant influence on the reflected and transmitted coefficients of structural surface and the energy reflected and transmitted coefficients (Chap. 7).

### **On Damage of Bedrock and Overburden Slope Landslide Induced by Earthquake**

- (7) On the basis of detailed research on landslide damage of Wenchuan earthquake, we focused on collecting and organizing the damage of bedrock and overburden slopes induced by earthquakes, and conducted statistical analysis results of correlation of the horizontal sliding distance  $L$  of 65 classic landslides with mountain height  $HL$ , tangent value of bevel angle  $\tan\theta$ , PGA, and landslide volume  $V$ , built the prediction model of horizontal sliding distance of bedrock and overburden slopes induced by Wenhcuau earthquake, and also

selected Donghekou large-scale landslide and other two commonly used sliding distance prediction model to verify the rationality and advantage of damage range prediction model of Wenchuan earthquake-induced bedrock and overburden slopes. The research results show:  $L$  is in positive correlation with PGA, HL, and  $V$ , and in negative correlation with  $\tan\theta$ ; the probability of landslide happening at  $1/3HL$  is 70.8 %, that of landslide happening at  $0.3\text{--}0.6HL$  is 15.4 %, and that of landslide happening at  $1/3HL$  is 13.8 %. Therefore, most landslides happen at  $1/3HL$  (Chap. 8).

(1)–(6) belong to research on micro-aspects, whose conclusion could be used to seismic design of concrete slope engineering; (7) belongs to macro-aspects, whose conclusion could be applied to decision on scheme of relocation and preventing collision, engineering management, and relocation range.

## Part Two: Creativity of This Dissertation

The creativity of this dissertation is as follows:

- (1) Tit proposed the double-sided rock slope acceleration elevation amplification effects time–frequency method, which can not only take good consideration of the influence of three essential elements (PGA, frequency, and duration) on acceleration elevation amplification, but also can provide theoretical supports to seismic design of rock and soil engineering in mountain regions in China (Chap. 5).

For slope deformation features and formation mechanism of bedrock and overburden slopes, this chapter conducted in-depth research from the perspective of joint time–frequency domain, promoted new analysis ideas to seismic mechanism of rock and soil engineering, which broke through the traditional analysis with sole reference to time-domain features or frequency-domain features (Chap. 6).

- (2) This chapter promoted seismic stability time–frequency analysis method of bedrock and overburden slope, which could consider the influence of three essential elements (PGA, frequency, and duration) of seismic wave on slope stability (Chap. 7),
- (3) This chapter promoted damage range prediction model of Wenchuan earthquake-induced bedrock and overburden slopes, which could consider four key factors including mountain height HL, tangent value of bevel angle  $\tan\theta$ , PGA, and landslide volume  $V$ , and it has high accuracy, and could provide scientific reference to decision on scheme of relocation and preventing collision, engineering management, and relocation range (Chap. 8).

### **Part Three: Problems and Prospects**

This dissertation conducted some beneficial exploration on the aspects of slope dynamic characteristics, dynamic response, seismic design method, landslide mechanism, stability evaluation method and landslide damage range evaluation, and obtained some creative results. But as stated in the beginning of this dissertation, the slope dynamic stability problem is a complex subject concerning multi-crossed disciplines. Due to limits of knowledge of the author and research conditions, there is still imperfection in this dissertation, and there are problems worth further study and discussion.

- (1) Conducting research on landslide damage mechanism, failure mode, motion features, and damage range;
- (2) Conducting further study on constitutive relation of rock and soil mass, and building correspondent constitutive relation model of rock and soil mass based on different materials and loading effects, which has important value to both theoretical analysis and engineering application, and is a key direction for research of slope dynamic stability analysis;
- (3) Conducting research on deformation mode and long-term evolutionary mechanism of rock slopes;
- (4) Conducting research on slope stability judging system based on nonlinear characteristics of slope structural surface;
- (5) Conducting research on theory of acceleration elevation amplification effects with full consideration of nonlinear characteristics of rock and soil mass.

# Bibliography

- Ai-Homoud AS, Tahtamoni WW. Reliability analysis of three-dimensional dynamic slope stability and earthquake-induced permanent displacement. *Soil Dyn Earthq Eng.* 2000;19(2):91–114.
- Ausilio E, Conte E, Dente G. Seismic stability analysis of reinforced slopes. *Soil Dyn Earthq Eng.* 2000;19(3):159–72.
- Bai GL, Xue F, Xu YZ. Seismic damage analysis and reduction measures of buildings in village and town in the Yushu Earthquake. *J Xi'an Univ Archit Technol (Natural Science Edition).* 2011;43(6):309–15.
- Biondi G, Cascone E, Maugeri M. Flow and deformation of sandy slopes. *Soil Dyn Earthq Eng.* 2002;22(10):1103–14.
- Bo JS, Xu GD, Jing LP. Seismic response and dynamic stability analysis of soil slopes. *Earthq Eng Eng Vib.* 2001;21(2):116–20.
- Bray JD, Repetto PC. Seismic design considerations for lined solid waste landfills. *Geotext Remembr.* 1994;13(8):497–518.
- Budetta P, De Riso R. The mobility of some debris flows in pyroclastic deposits of the northwestern Campanian region (southern Italy). *Bull Eng Geol Environ.* 2004;63:293–302.
- Cai SH, Wang LM, Yuan ZX. A Preliminary study on the seismic landslide distance in the shanxi-gansu-ningxia-shanxi Loess region. *Northwest Seismol J.* 1998;20(4):75–82.
- Celebi M. Topographic and geological amplification determined from strong-motion and aftershock records of 3 March 1985 Chile earthquake. *Bull Seismol Soc Am.* 1987;77:1147–57.
- Chen XL, Ran HL, Wang MM. Hazards zonation for potential earthquake-induced landslide area. *Chin J Geophys.* 1999;35(6):24–5.
- Chen ZY, Mi HL, Wang XG. A three-dimensional limit equilibrium method for slope stability analysis. *Chin J Rock Mech Eng.* 2001;23(5):525–9.
- Chen YM, Ke H, Ling DS. Dynamic properties and seismic response of municipal solid waste. *China Civ Eng J.* 2002;35(3):66–72.
- Chen LL, Chen MZ, Qian SG. Stability analysis of high-steep rocky slope under earthquake loads. *J Yangtze River Sci Res Inst.* 2004;21(1):33–5.
- Clough RW, Chopra AK. Earthquake stress analysis in earth dams. *J Eng Mech, ASCE.* 1966;92 (EM2).
- Crespellani T, Madiari C, Vannucchi G. Earthquake destructiveness potential factor and slope stability. *Geo-technique.* 1998;48(3):411–9.
- Cui P, Wei FQ, Chen XQ. Geo-hazards in Wenchuan Earthquake Area and Countemeasures for Disaster Reduction. *Bull Chin Acad Sci.* 2008;23(4):317–23.
- Devoli G, De Blasio FV, Elverh A. Statistical analysis of landslide events in Central America and their run—out distance. *Geotech Geol Eng.* 2009;27:23–42.
- Ding YH, Wang YQ, Sun JZ. Correlation between landslides and seislides and seismic parameters and its application in predicting slope earthquake disaster. *Chin J Geophys.* 1999;42 (Suppl):101–7.

- Ding YH, Wang YQ, Sun JZ, Tang Y. Research on the method for prediction of earthquake-induced landslides and its application to engineering projects. *J Eng Geol.* 2000;8(4):475–80.
- Du XL, Han JY, Li LY. Selection of shaking table test similarity Relations for Long-distance Buried Pipeline. *J Disaster Prev Mitig Eng.* 2013;33(3):246–52.
- Duncan JM. State of the art: limit equilibrium and finite-element analysis of slopes. *J Geotech Eng.* 1996;22(7):577–96.
- Fan XY, Qiao JP. Influence of landslide and ground factors on large-scale landslide movement. *Chin J Rock Mech Eng.* 2010;29(11):2337–47.
- Feng WK, Xu Q, Huang RQ. Preliminary study on mechanical mechanism of slope earthquake-induced deformation. *Chin J Rock Mech Eng.* 2009;28(Suppl):3124–30.
- Hattanjil T, Moriwaki H. Morph metric analysis of relic landslides using detailed landslide distribution maps: implications for forecasting travel distance of future landslides. *Geomorphol.* 2009;103:447–54.
- Hartzell SH, Carver DL, King KW. Initial investigation of site and topographic effects at Robin wood ridge, California. *Bull Seis Soc Am.* 1994;84:1336–49.
- Hong YS, Chen RH, Wu CS. Shaking analysis of steep nailed slopes. *Can Geotech J.* 2005;42(5):1264–79.
- Hu YX, Zhang YS, Liang JW, HHT-based identification of site liquefaction. *China Civ Eng J.* 2006;39(2):66–77.
- Huang NE, Shen Z, Long SR. The empirical mode decomposition and Hilbert spectrum for nonlinear and non-stationary time series analysis. *Proc R Soc Lond.* 1998; 454:903–95.
- Huang RQ, Li WL. Research on development and distribution rules of geohazards induced by Wenchuan earthquake on 12th May, 2008. *Chin J Rock Mech Eng.* 2008;27(12):2585–92.
- Hungr O. A model for the run-out analysis of rapid flow slides, debris flows, and avalanches. *Can Geotech J.* 1995;32:610–23.
- Jiang XY, Qiao JP. Contact mechanics model for risk prediction of typical landslides. *Eng Mech.* 2006;23(8):106–9.
- Keefer DK. Landslides caused by earthquake. *Bull Geol Soc Am.* 1989;95:406–21.
- Keefer DK. Statistical analysis of an earthquake-induced landslide distribution—the 1989 Loma Prieta, California event. *Eng Geol.* 2000;58(3–4):231–49.
- Kurita RR, Rodriguez-Ovejero LA. Model with non-reflecting boundaries for use in explicit soil-structure interaction analyses. *Earthq Eng Struct Dyn.* 1980;8:361–74.
- Lam L, Fredlund DG. A general limit equilibrium model for three-dimensional slope stability analyses. *Can Geotech J.* 1993;30(6):905–19.
- Lee KL. Seismic permanent deformations in earth dams. Los Angeles: School of Engineering and Applied Science, University of California. 1974.
- Leshchinsky D, Ching SK. Pseudo-static stability of slopes: Design. *J Geotech Eng ASCE.* 1994;120(9):1514–32.
- Li SH, Liu TP, Liu XY. Analysis method for landslide stability. *Chin J Rock Mech Eng.* 2009;28:3309–24.
- Lin JS, Whitman R. Earthquake induced displacements of sliding blocks. *J Geotech Eng.* 1986;112(1):44–59.
- Lin M.-L, Wang K.-L. Seismic slope behavior in a large-scale shaking table model test. *Eng Geol.* 2006;86(2–3):118–33.
- Ling HI, Cheng AD. Rock sliding induced by seismic force. *Int J Rock Mech Min Sci.* 1997;34(6):1021–9.
- Liu LP, Lei ZY, Zhou FC. The evaluation of seismic slope stability analysis methods. *J Chongqing Jiaotong Univ.* 2001;20(3):83–88.
- Liu HS, Bo JS, Liu DS. Review on study of seismic stability analysis of rock-soil slopes. *Earthq Eng Vib.* 2005;25(1):164–71.
- Liu HS, Bo JS, Liu DD. Development on study of seismic stability evaluation methods of rock-soil slopes. *J Inst Disaster Prev Sci Technol.* 2007;9(3):20–27.

- Lu YX, Shi YC, Chen YM. Slippage estimation of the loess landslide triggered by earthquake. *Northwest Seismol J.* 2006;28(3):248–51.
- Lu M, Li XJ, An XW. A comparison of recorded response spectra from the 2008 Wenchuan, China, earthquake with modern ground-motion prediction models. *Bull Seismol Soc Am.* 2010;100(5B):2357–80.
- Lveda SA, Sepu A, Murphy W, Randall C, Jibson W, et al. Seismically induced rock slope failures resulting from topographic amplification of strong ground motions: The case of Pacoima Canyon, California. *Eng Geol.* 2005;80(3–4):336–48.
- Men YM, Peng JB, Li XC. Research on vibration testing of models for dynamic stability of rock slope with layered structures. *World Earthq Eng.* 2004;20(4):131–36.
- Mu CX, Yan WM, Zhou Q. The application progress of horizontal isolation devices for museum free-standing cultural relics. *J Water Resour Archit Eng.* 2014;5(2):1–6.
- Newmark NM. Effects of earthquakes on dams and embankments. *Geo-technique.* 1965;15(2):139–60.
- Okura Y, Kitahara H, Sammori T. The effects of rock-fall volume on run-out distance. *Eng Geol.* 2000;58:109–24.
- Qi SL, Qi SW, Wu FQ. On permanent displacement of earthquake induced slide based on residual pushing force method. *J Eng Geol.* 2004;12(1):63–8.
- Qi SW, Wu FQ, Liu CL, Ding YH. Engineering geology analysis on stability of slope under earthquake. *Chin J Rock Mech Eng.* 2004;23(16):2792–7.
- Qiao JP, Pu XH, Wang M. A study on characteristics of distribution of earthquake-induced landslides and hazard zoning. 2009;24(21):25–9.
- Qu HL, Zhang JJ, Wang FJ. Seismic response of prestressed anchor sheet pile wall from shaking table tests. *Chin J Geotech Eng.* 2013;35(2):313–20.
- Rodriguez CE, Bommer JJ, Chandler RJ. Earthquake-induced landslides: 1980–1997. *Soil Dyn Earthq Eng.* 1999;18:325–46.
- Sakai H, Sawada S, Toki K. Structure considering tensile failure. 12WCEE, 2000, Paper NO. 678.
- Scheidegger AE. On the prediction of the reach and velocity of catastrophic landslide. *Rock Mech.* 1973;5:231–6.
- Seed HB, Lee KL, Idriss IM. Analysis of the slides in the San Fernando dams during the earthquake of Feb. 9, 1971. Berkeley: EERC, University of California. 1973.
- Shi C, Zhou JW, Ren Q, Zhou XQ. Ray theory solution of the elevation amplification effect on a single-free-face slope. *J Hohai Univ.* 2008;36(2):238–41.
- Siad L. Seismic stability analysis of fractured rock slopes by yield design theory. *Soil Dyn Earthq Eng.* 2003;23(3):203–12.
- Siyahi BG. Pseudo-static stability analysis in normally consolidated soil slopes subjected to earthquake. *Teknik Dergi/Tech J Turkish Chamber Civil Eng.* 1998;9(DEC):457–61.
- Sun CS, Cai HW. Analysis of landslides triggered by Wenchuan earthquake, seismology and geology. 1997;6(1):25–30.
- Sun P, Yin YP, Wu SR, Chen LW. Experimental study Of microstructure and mechanical properties of rocks from Donghekou landslide. *Chin J Rock Mech Eng.* 2010;29(1):2872–78.
- Tao LJ, Su SR, Zhang ZY. Dynamic stability analysis of jointed rock slope. *J Eng Geol.* 2001;32–8.
- Wang, FW, Sassa K. A modified geotechnical simulation model for landslide motion. *Landslides-Proceedings of 1st European Conference Landslides.* Prague, June 2002, in Press.
- Wang HP, Li SC, Zhang QY, Li Y, Guo XH. Development of a new geomechanical similar material. *Chin J Rock Mech Eng.* 2006;25(9):1842–7.
- Wang YH, Cheng WR. Dynamic property of a shaking table simulating earthquake. *J Vib Shock.* 2010;29(2):99–103.
- Wang HY, Xie LL. Effects of topography on ground motion in the Xishan park, Zigong city. *Chin J Geophys.* 2010;53(7):1631–8.
- Wang NQ, Zhang ZY, Wang JD. Forecasting method of sliding distance on typical loess landslides. *J Northwest Univ (Natural Science Edition).* 2003;33(1):111–4.

- Wu XY, Law KT, Selvadurai A.P.S. Examination of the pseudo-static limit equilibrium method for dynamic stability analysis of slopes. Canadian: Canadian Geotechnical Conference. 1991.
- Wu ZY, Bao JS, Liu HS. A method for evaluating dynamic safety factor rock slope seismic stability analysis. *J Disaster Prev Mitig Eng.* 2004;24(3):228–41.
- Wu SC, Zhang XP, Liu Y. Analysis of failure process of similar soil slope with weak intercalated layer based on particle flow simulation. *Rock soil Mech.* 2008;29(11):2989–904
- Xie H, Wang SG, Kong JM. Distribution and characteristics of mountain hazards induced by the earthquake of May 12 in Wenchuan, China. *J Mt Sci.* 2005;26(5):396–401.
- Xin HB, Wang YQ. Earthquake induced landslide and avalanche. *Chin J Geotech Eng.* 1999;21(5):591–4.
- Xu Q, Huang RQ. Kinetics characteristics of large landslides triggered by May 12th Wenchuan earthquake. *J Eng Geol.* 2008;16(6):721–9.
- Xu GX, Yao LK, Gao ZN, Li ZH. Large-scale shaking table model test study on dynamic characteristics and dynamic responses of slope. *Chin J Rock Mech Eng.* 2008;27(3):624–32.
- Xu Q, Chen JJ, Feng WK. Study of the seismic response of slopes by physical modeling. *J Sichuan Univ.* 2009;43(3):262–6.
- Xu Q, Pei XJ, Huang RQ. Large-scale landslides induced by the Wenchuan earthquake. Beijing: Science Press; 2009.
- Xu Q, Dong XJ. Genetic types of large-scale landslides induced by Wenchuan earthquake. *Earth Sci-J China Univ Geosci.* 2011;36(6):1134–42.
- Yang CW. Study on seismic dynamic characters of rock slopes and system including formation mechanism of landslides, stability discrimination of slope and assesment of hazard scope of landslide for slope of bedrock and overburden layer. Doctoral dissertation. Southwest Jiaotong University; 2010.
- Yang CW, Zhang JJ, Zhou DP. Research of time-frequency analysis method for seismic stability of rock slope subjected to sv wave. *Chin J Rock Mech Eng.* 2013;32(3):483–91.
- Yang CW, Zhang JJ. Landslide responses of high steep hill with two-side slopes under ground shaking, *J Southwest Jiaotong Univ.* 2013;48(3):415–22.
- Yin YP. Features of landslides triggered by the Wenchuan earthquake. *J Eng Geol.* 2009;17(1):29–38
- Zhang CH, Pekau OA, Jin F. Application of distinct element method in dynamic analysis of high rock slopes and blocky structures. *Soil Dyn Earthq Eng.* 1997;16(1):385–94.
- Zhang P, Chen XM, Wang XD. Analysis of near-fault ground motion and seismic landslide failure mode in Wenchuan earthquake. *J Nanjing Univ Technol (Natural Science Edition).* 2009;31(1):49–55.
- Zhang JJ, Han PF. Displacement-based aseismic design method for gravity retaining walls-large scale shaking table tests. *Chin J Geotech Eng.* 2012;34(3):416–23.
- Zhang JJ, Liao Y, Qu HL. Seismic damage of earth structures of road engineering in the 2008 Wenchuan earthquake. *Environ Earth Sci.* 2012;65:987–93.
- Zhang JJ, Yang CW, Zhao J, Graeme HM. Empirical models for predicting lateral spreading with considering the effect of region seismicity. *Earthq Eng Eng Vib.* 2012;11(1):121–31.
- Zhao J X, Zhang J, Asano A. Attenuation relations of strong ground motion in Japan using site classification based on predominant period. *Bull Seismol Soc Am.* 2010;100(5B):2357–80.
- Zheng YR, Ye HL, Huang RQ. Analysis and discussion of failure mechanism and fracture surface of slope under earthquake. *Chin J Rock Mech Eng.* 2009;28 (8):1714–23.
- Zhong DH, An N, Li MC. 3d Dynamic simulation and analysis of slope instability of reservoir banks. *Chin J Rock Mech Eng.* 2007;26(2):360–7.