



Review Paper

Slope stability analysis using recent metaheuristic techniques: a comprehensive survey

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Abstract

In the framework of designing civil engineering structures such as dams and road embankments, slope stability assessment is essential. Many slope stability assessment methods based on swarm intelligence algorithms and artificial intelligence techniques have been developed in the past decade. Therefore, this paper aims to provide an up-to-date overview of slope stability by summarizing the review of applications of several metaheuristics in this field with their advantages and limitations. In this study, we present recent swarm intelligence methods and machine learning techniques used for assessing the stability of slopes and compare them with the antlion optimiser (ALO) technique. The factor of safety related to every slip surface searched was computed using Morgenstern-Price method. The performance of the proposed ALO is evaluated and validated for four slope examples against recent metaheuristics. Finally, the work carried out will help practitioners as they can now have all swarm intelligence approaches combined in one paper and will help in choosing them the right technique based on their application.

Keywords Factor of safety · Metaheuristic algorithms · Artificial intelligence · Optimisation techniques · Limit equilibrium method

1 Introduction

Slope stability assessment is a crucial step in civil engineering for designing earth dams and road embankments. The computation of the minimum factor of safety (FS) is paramount to slope assessments because misjudgements may cause catastrophic slope failures and loss of life as reported in previous studies [1–4]. A FS higher than 1 means that the slope is stable but normally in engineering applications a minimum value of 1.5 is required for static gravity loading and between 1.2–1.3 for seismic conditions particularly when slope failure causes major damages such as slopes beneath bridge abutments, major roadways and retaining structures. Generally, for slope stability assessment several trial slip surfaces are considered to determine

the one with the minimum FS value. The identification of the critical slip surface is the primary step, which is regularly performed using traditional limit equilibrium methods (LEM) in professional practice. Recently, bio-inspired optimisation techniques have made assessing slope stability easier, particularly for heterogeneous slopes and slopes with a weak band of a soil layer sandwiched between two strong layers [5]. In cases involving the aforementioned slope conditions, the problem to be analysed is nonlinear with a noncircular slip surface, whereas the search requires more than three variables. The three variables for the circular slip surface are coordinates of the center of the slip surface (horizontal, vertical) and radius of the slip surface.

Several techniques, including prominent traditional techniques to recent optimisation techniques, used to

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assess slope stability are summarised in this paper. The most prevalently used method for performing slope stability calculations is traditional limit equilibrium method [6–11]. Classical techniques for assessing slope stability are unsuitable for several cases such as slopes with non-circular slip surfaces having multiple local minima solution and require a close by preliminary solution to obtain a valid result. A traditional LEM can identify the critical slip surface; however, the method fails to identify a global minimum in certain situations causing premature convergence at a local minimum [12]. Furthermore, other techniques, including finite difference [13] and finite element strength reduction techniques [14–17] are extensively used. By contrast, finite element techniques need not assume the pattern and position of trial slip surfaces and introduce other complexities, such as defining stress-strain relations of soils. Monte Carlo methods used for slope stability calculation [18, 19] should have quality inputs to arrive to a desirable FS and they often underestimate the probability of slope failures. Furthermore, MC methods are slow in computation as large sample size is required to obtain desirable result.

At present, intelligent algorithms such as harmony search (HS) [20], ant colony optimisation (ACO) [21–23], particle swarm optimisation (PSO) [24, 25], simulated annealing (SA) [26], artificial bee colony (ABC) [27], cuckoo search (CS) [28], firefly algorithm (FA) [28], fish swarm optimisation (FSO) [29], gravitational search algorithm (GSA) [30–32], big bang - big crunch (BB-BC) optimisation [33], relevance vector machine [34], mutative scaled chaos (MSC) [35], tabu search (TS) [36], genetic algorithms (GA) [37–42], fireworks algorithm (FWA) [43], black hole algorithm (BHA) [44], immunised evolutionary programming (IEP) [45], differential evolution (DE) [46, 47], evolutionary strategy (ES) [46, 47], and biogeography-based optimisation (BBO) [46, 47], imperialistic competitive algorithm (ICA) [48], multiverse optimisation algorithm (MVO) [49] and teaching-learning-based optimisation (TLBO) [50] have been applied for slope stability analysis. Moreover, the hybridisation of these algorithms, such as CS with boundary constraint (CS-EB) [51], PSO with HS (PSO-HS) [52], ACO with simulated annealing (ACO-SA) [53], and GSA with sequential quadratic programming (GSA-SQP) [54] are used. Furthermore, artificial neural networks (ANNs) [55–64], reliability index [65] and fuzzy logic [66] have also been used to analyse the stability of slopes and earth dams. These studies indicate the potential of metaheuristic algorithms for assessing slope stability.

The FS results obtained using these metaheuristic approaches get trapped in local optimum because of the selection of an inaccurate set of parameters. For e.g., ACO has limitations because it may converge to local optimum providing inaccurate results, which is attributed

to the selection of inaccurate tuning parameters, and requires high computation time. A similar case used GA for optimisation because it required the careful selection of crossover and mutation rates. Additionally, PSO and ABC use inertia weights, social and cognitive parameters, and the number of bees and limits, respectively. Hybrid approaches require complex actions, such as hybridising, which are difficult to include for practitioners. In addition, the ANN has severe limitations because it requires a large database comprising several slope failure conditions and can only be applied to the considered case. Artificial intelligence (AI) techniques [67] utilised for accurate prediction of slope stability require hyperparameter and weight tuning. Table 1 summarises different algorithmic specific parameters and their numerical values tuned for computing FS of slopes. Thus, all optimisation techniques excluding BHA and TLBO, require careful parameter tuning to capture global optima. Hence, different metaheuristic approaches that do not encounter the aforementioned limitation of parameter tuning are required, which can evade this local minima in most cases.

The no-free-lunch theorem [68] entices scientists to continually come up with novel metaheuristic algorithms and employ them to their respective fields of interest such as optimizing the mechanical stabilization of earth walls [69] and reinforced concrete cantilever retaining walls [70]. In 2015, Mirjalili [71] proposed a new optimisation method termed ALO whose description is provided in Sect. 2. Several studies [72, 73] have compared ALO with GA and PSO and indicated the superior performance of ALO in solving non-convex and complex optimisation problems. The ALO algorithm has been successfully applied to solve numerous engineering problems [72–79]. In this particular application, a procedure for employing an ALO method was developed and adapted for calculating the minimum FS of a slope, and four benchmark slopes were considered. A comparison between different results obtained using other state-of-the-art approaches and the current technique indicates advantages and limitations of the new method. Different algorithms may have different fitness function evaluations consumed per iteration [80, 81], and therefore, the comparison of FS with other metaheuristics approaches is considered.

The paper is organised as follows: Sect. 1 introduces the previous studies performing geotechnical slope stability calculations by using intelligent algorithms and limitations of traditional techniques; Sect. 2 introduces the ALO algorithm; Sect. 3 discusses slope stability calculations of the ALO algorithm; in Sect. 4, four case studies are presented to validate the technique; and finally, in Sect. 5 conclusions are presented.

Table 1 Literature review on algorithmic specific parameters used in ALO and other metaheuristic approaches for slope stability analyses

Algorithm	Algorithmic specific parameters for each algorithm with their numerical values used in slope stability analysis by different researchers
ACO [21]	Ant colony size (300), total number of tours (200), pheromone evaporation rate ($\rho = 0.3$), bilinear scaling constant $Z = 2$ for the quality function
PSO [24]	Inertial weight $\omega = 0.5$, swarm size (60), cognitive parameter $c_1 = 2$ and social parameter $c_2 = 2$, number of iterations (200)
SA [26]	Initial temperature state, cooling rate, acceptance probability, maximum number of iterations
ABC [27]	Bee colony size (20–50), number of cycles (100), limit (300–650)
FA [28]	Swarm of $n = 50$ particles, parameter β representing attractiveness, light absorption coefficient (γ) varying between 0.1 and 10, generation = 3000;
FSO [29]	Fish pool $f_p = 0.9$, number of fishes in f_p , parameter for optimisation $pr = 0.1$, probability array $\chi = 0.8$, maximum number of iterations = 700
GSA [30, 31]	Population size $N = 50$, initial gravitational constant $G_0 = 100$, constant $\beta = 0.1$, maximum iteration $t_{max} = 1000$
BBO [32]	Number of habitats (100), mutation rate $M_{max} = 0.2$, maximum immigration and emigration rate $I = 0.5$ and $E = 1$, number of generations (250)
BB-BC [33]	Universe composed of $N_b = 30$ number of bodies, search space reduction parameter $\delta = 0.7–0.9$, maximum size of the step $\theta = 1$, scaling factor $\zeta = 0.7$, generation number = 50, distribution constants $\alpha = 1.4$, $\beta = 0.3$
GA [37]	Crossover (0.75) and mutation probability (0.002), position of crossover, length of chromosome (24), population size (200), number of generations (300)
FWA [43]	Number of spark seeds $N = 10$, number of generating sparks $M = 40$, maximum explosion amplitude $\hat{A} = 40$, number of Gaussian sparks $M_e = 5$, total number of iterations (2000)
BHA [44]	Population of stars (50), maximum number of iterations (500)
DE [46]	Weighting factor F , crossover constant C_r , size of population (50), maximum number of generations (3000)
ES [46]	Size of population (50), maximum number of generations (3000), number of offspring λ , standard deviation σ
ICA [48]	Population of countries (300), population of imperialists (8) and decades (500), rate of revolution (0.3), damp ratio (0.99), uniting threshold (0.02), control parameter $\alpha = 0.1$, $\beta = 2$, $\gamma = 0.02$
HS [52]	Harmony memory consideration rate $HR = 0.98$, the pitch adjustment rate $PR = 0.1$, harmony memory HM of size M , number of function evaluations $NOFs$
TLBO [50]	Number of learners and maximum number of iterations
ALO (proposed approach)	Population of ants and antlions (10–100), maximum number of iterations (100)

Acronyms of all algorithms used are reported in Sect. 1

2 Overview of ALO algorithm

ALO is a bio-inspired optimisation algorithm popularised by Mirjalili [71]. It is a swarm-based algorithm, which employs random walks and roulette wheel selections biased using fitness functions for selecting control

variables, and therefore, probability of falling for local minima is considerably low. The five primary steps involved in ALO are as follows: First, an antlion creates a cone-shaped sandpit (Fig. 1a). The antlion then hides from the ant (Fig. 1b, c) and waits for the ant to get trapped. When the ant enters cone-shaped sandpit, the antlion tosses sand outside the pit, compelling the ant

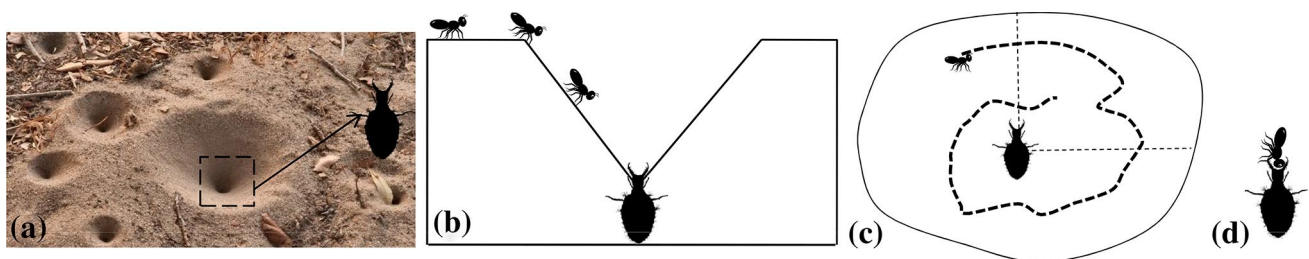


Fig. 1 **a** Cone-shaped trap of antlion with antlion hiding beneath the pit [78], **b** ants entering the trap [78], **c** random walk of ant in antlion's pit, and **d** ant being consumed by antlion

to fall towards the edge of the pit. The ant is then pulled into the sandpit and consumed (Fig. 1d). Finally, the antlion throws the uneaten ant and plans for the next plot. The mathematical model of ALO is briefly explained as follows [71, 76, 78]:

2.1 Initialisation

The ALO algorithm models the interaction of two populations, namely ants and antlions. The population of ants is equal to number of antlions, denoted by constant NP . The position of the ants (Eq. 1) is stored in matrix Mat_{Ant} where each row represents the values of control variables for a particular solution.

$$Mat_{Ant} = \begin{pmatrix} 1 & 2 & \dots & Dim \\ Ant_{1,1} & Ant_{1,2} & \dots & Ant_{1,Dim} \\ Ant_{2,1} & Ant_{2,2} & \dots & Ant_{2,Dim} \\ \vdots & \vdots & \vdots & \vdots \\ Ant_{NP,1} & Ant_{NP,2} & \dots & Ant_{NP,Dim} \end{pmatrix} \begin{matrix} 1 \\ 2 \\ \vdots \\ NP \end{matrix} \quad (1)$$

where Ant_{ij} and Dim indicate the position of the j th control variable for the i th ant and the total number of control variables, respectively. Mat_{OAnt} (Eq. 2) and Fit are the matrix storing the fitness function associated with each ant position and the fitness function, respectively.

$$Mat_{OAnt} = \begin{pmatrix} Fit(Ant_{1,1}, Ant_{1,2}, \dots, Ant_{1,Dim}) \\ Fit(Ant_{2,1}, Ant_{2,2}, \dots, Ant_{2,Dim}) \\ \vdots \\ Fit(Ant_{NP,1}, Ant_{NP,2}, \dots, Ant_{NP,Dim}) \end{pmatrix} \begin{matrix} 1 \\ 2 \\ \vdots \\ NP \end{matrix} \quad (2)$$

Moreover besides the ants, the antlions also hide whose matrices $Mat_{Antlion}$ and $Mat_{OAntlion}$ are used to store their scavenging positions and fitness values. After the initialisation step, the optimal antlion (antlion with the maximum fitness value) is selected as an elite.

2.2 Digging traps

The ALO algorithm utilises a roulette wheel mechanism to select antlions based on their fitness values. This process ensures high probability for ensnare ants for antlions with high fitness. For each ant, one antlion is selected, thus only one ant is trapped by one antlion.

2.3 Sliding ants towards antlion

Antlions start tossing sand away from the pit after realising that the ant is entering the trap. Thus boundaries of ants random walk (Fig. 1c) are updated as below:

$$c_i(Iter) = \frac{c_i(Iter)}{10^{\omega \frac{Iter}{Iter_{max}}}} \quad (3)$$

$$d_i(Iter) = \frac{d_i(Iter)}{10^{\omega \frac{Iter}{Iter_{max}}}} \quad (4)$$

where $c_i(Iter)$ and $d_i(Iter)$ are the vector containing minimum and maximum limits of the i th control variable, and $Iter$ denotes the present iteration number and ω , a constant based on range of ratio of $Iter$ and $Iter_{max}$ (maximum number of iterations) [71].

2.4 Entrapment in antlion pits

The subsequent equations below model the entrapment of ants in antlion pits, thus influencing their random walks:

$$c_i^m(Iter) = Antlion_j(Iter) + c_i(Iter) \quad (5)$$

$$d_i^m(Iter) = Antlion_j(Iter) + d_i(Iter) \quad (6)$$

2.5 Random walk of ants in antlion pit

In this step, the population of ants (Fig. 1b) is initialised using a random walk by the following equation:

$$P(Iter) = (0, cums(2rw_1 - 1), cums(2rw_2 - 1), \dots, cums(2rw_{Iter_{max}} - 1)) \quad (7)$$

where $cums$ provides accumulative sum, and rw_j comprises of the j th row of the vector, whose value is 1 for random number generated greater than 0.5 and 0 otherwise. The walks are normalised by the following equation to keep them within bounds.

$$P_i(Iter) = \frac{(P_i(Iter) - a_i)(d_i(Iter) - c_i(Iter))}{(b_i - a_i)} + c_i(Iter) \quad (8)$$

where $P_i(Iter)$ is the updated using normalised position of the i th control variable at the $Iter$. a_i and b_i are the lower and upper limits of the i th control variable, while the indices for $c_i(Iter)$ and $d_i(Iter)$ denote the lower and upper boundaries for the current iteration $Iter$.

2.6 Elitism

Parameters of the ant are now calculated based on the current optimal antlion and global optimal antlion to enhance

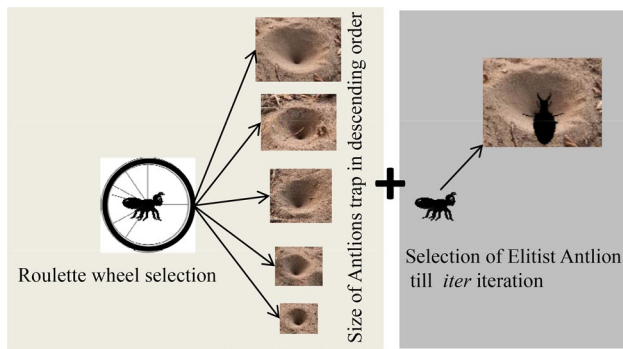


Fig. 2 Figure illustrating the update of ants position based on roulette wheel mechanism and elite antlion

the local and global search capability (Fig. 2). The position of the *i*th ant at current iteration is updated as follows:

$$Ant_i(Iter) = \frac{RW_{Roulette}(Iter) + RW_{Elite}(Iter)}{2} \tag{9}$$

where $RW_{Roulette}(Iter)$ and $RW_{Elite}(Iter)$ are random walks of the *i*th ant around antlion tabbed through roulette wheel mechanism and the elite antlion.

2.7 Catching preys, rebuilding traps

The fitness function for the position of the ant is calculated in this step. The fittest solution is replaced with the existing solution if it is superior ($fit(Ant_i(Iter)) < fit(Antlion_j(Iter))$) else the previous solution is retained as per the following equation:

$$Antlion_j(Iter) = Ant_i(Iter) \tag{10}$$

The aforementioned steps in Sects. 2.2–2.7 are repeated until $Iter_{max}$ are reached.

3 Implementation of ALO to slope stability problem

The method explained in Sect. 2 is modified to fit into a slope stability problem. The process comprises of the following steps: the generation of the trial slip surface, calculating the FS, and using ALO to identify the critical slip surface and compute its FS. A computer code is developed in MATLAB [82] to automatically search the critical slip surface using ALO.

3.1 Generation of trial slip surface and FS computation

Before calculating for the FS, the slip surface must be generated using a slip surface generation algorithm. Several such algorithms are available [20]. In this paper, the method proposed by Cheng [83] is used. The method by Cheng [83] is used as it was easy to implement in computer programming language and involved fewer ranges to be defined for control variables (only ranges of entry and exit points of slip surfaces). Figure 3 illustrates the method of generating the trial slip surface, where $y = G(x)$ and $B(x)$ represents the mathematical function of slope geometry and the equation of the bedrock, respectively.

In the first step, the failure mass is split into *N* vertical segments having uniform width. Although from previous researches, horizontal slice methods developed by Lo and Xu [84] are having many advantages in solving for the safety factor of slopes, but for the scope of work only vertical slices are used in the present study. The failure surface is defined by *N* + 1 vertices $[V_1, V_2, \dots, V_{N+1}]$ as follows:

$$V = [(x_1, y_1), (x_2, y_2), \dots, (x_N, y_N), (x_{N+1}, y_{N+1})] \tag{11}$$

Values of x_1 (entry) and x_{N+1} (exit) shown in Fig. 4a can be determined from engineering experience. Because all slices have the same width, x-coordinates of points from x_2 to x_N (not control variables) can be determined by the following equation:

$$x_{i+1} = x_1 + \left(\frac{x_{N+1} - x_1}{N} \right) \times i \text{ for } i = 1, 2, \dots, N - 1, N \tag{12}$$

For *y* coordinates y_1 and y_{N+1} can be computed using the geometry of the slope. To determine *y* coordinates of other points, upper and lower bounds (y_{imax} and y_{imin}) that represent slope geometry and bedrock profile, respectively, are considered. For optimisation algorithms, minimum and maximum limits of decision variables are pre-stated and constant during the optimisation. Lower and upper bounds of ordinates of vertices from V_2 till V_N are set to y_{imin} and y_{imax} because their minimum and maximum value are dynamically defined using the following equation:

$$y_i = y_{imin} + (y_{imax} - y_{imin}) \times \sigma_i; i = 1, \dots, N - 1 \tag{13}$$

To summarise the optimisation process for identifying the critical slip surface having a minimum FS can be summarised as follows:

$$\begin{aligned} \min f(x \leftarrow \mathbf{V}) \text{ s.t } x_l \leq x_1 \leq x_u; x_L \leq x_{N+1} \leq x_U; \\ 0 < \sigma_i < 1; i = 1, \dots, N - 1 \end{aligned} \tag{14}$$

where **V** can be obtained using the aforementioned procedure.

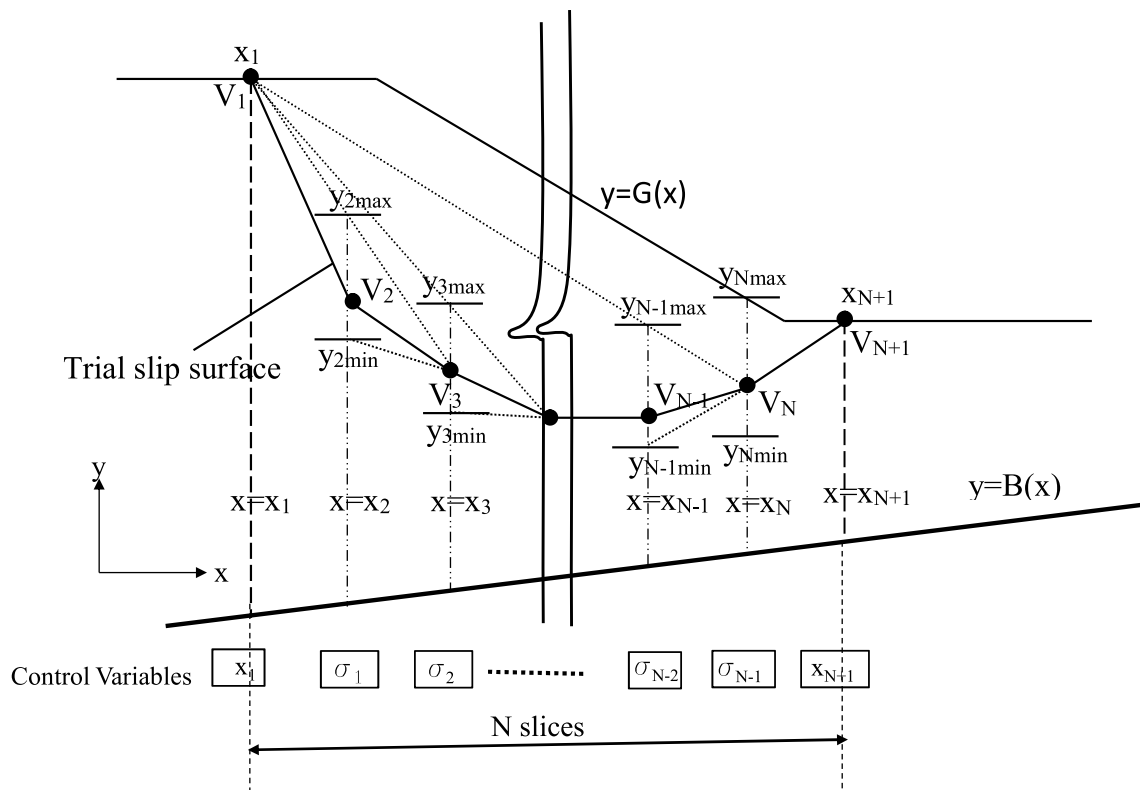


Fig. 3 Procedure for generating trial slip surfaces (modified from [83])

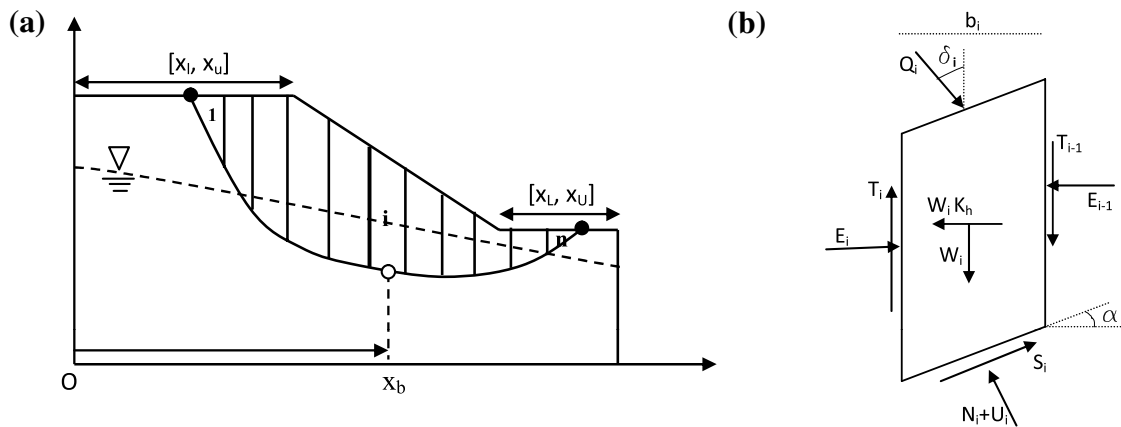


Fig. 4 a Sliding body showing division of slices and b forces acting on *i*th slice

For each trial slip surface generated by aforementioned procedure, Morgenstern-Price method [8] is used to compute the FS. However, equilibrium equations for force and moment summing over all slices are nonlinear and cannot be solved using a simple technique. In this paper, the nonlinear equations are solved using a numerical procedure developed by Zhu et al. [85] to obtain converging values for the FS and scaling factor (λ). In Morgenstern-Price method, the

ratio of interslice forces (T_i/E_i) is assumed to be in a $\lambda f(x_b)$, and the relation can be expressed as follows:

$$\frac{T_i}{E_i} = \lambda f(x_b) \tag{15}$$

where T_i and E_i are shear interslice and normal interslice forces, respectively (Fig. 4b). $f(x_b)$ is the interslice force function that varies with respect to x_b (Fig. 4a).

The sliding mass is split into vertical slices and resolved into forces as shown in Fig. 4a to evaluate the FS. From Fig. 4b, where W_i = weight of i th slice; U_i = pore water pressure at the base of the i th slice; N_i = effective normal force at the base of the i th slice; Q_i = external surcharge load acting on the i th slice at inclination δ_i , α_i = horizontal angle of the slice base; b_i = the width of the i th slice; K_h = horizontal seismic coefficient; and E_i and E_{i-1} are the normal interslice forces acting on left and right boundaries of the slice, respectively. For soil parameters, c' , ϕ' = effective cohesion and internal friction angle at the base of the i th slice. The following iterative algorithm is employed to evaluate the FS [85]:

- i) Compute R_i and T_i for each slice by using the following equations:

$$R_i = (W_i \cos \alpha_i - K_h W_i \sin \alpha_i - u_i b_i \sec \alpha_i) \tan \phi'_i + c_i b_i \sec \alpha_i \tag{16}$$

$$T_i = W_i \sin \alpha_i + K_h W_i \cos \alpha_i \tag{17}$$

- ii) The initial trial values of the FS and λ must be selected through criteria [85], however $FS_o = 1$ and $\lambda_o = 0$ can be safely selected as an initial prediction.
- iii) A constant interslice function is selected; that is, interslice forces are parallel, which is a particular case of Morgenstern-Price method, equivalent to Spencer method [86].

$$f(x_b) = 1 \tag{18}$$

- iv) Compute coefficients Φ_i and ψ_{i-1} for each slice by using the following equations:

$$\Phi_i = (\sin \alpha_i - \lambda f_i \cos \alpha_i) \tan \phi'_i + (\cos \alpha_i - \lambda f_i \sin \alpha_i) FS \tag{19}$$

$$\psi_{i-1} = \frac{\Phi_i}{\Phi_{i-1}} \tag{20}$$

- v) The FS is calculated using Eq. 21 and Φ_i and ψ_i are then determined using the current value of the FS. The FS is recomputed using updated values of Φ_i and ψ_i .

$$FS = \frac{\sum_{i=1}^{n-1} (R_i \prod_{j=1}^{n-1}) + R_n}{\sum_{i=1}^{n-1} (T_i \prod_{j=1}^{n-1}) + T_n} \tag{21}$$

- vi) Use Eqs. 22 and 23 to calculate E_i and λ .

$$E_i = \frac{\psi_{i-1} E_{i-1} \Phi_{i-1} + FS \times T_i - R_i}{\Phi_i} \tag{22}$$

when E_o and E_n are the interslice force at boundaries and are set at 0.

$$\lambda = \frac{\sum_{i=1}^n [b_i(E_i + E_{i-1}) \tan \alpha_i + K_h W_i h_i]}{\sum_{i=1}^n [b_i(f_i E_i + f_{i-1} E_{i-1})]} \tag{23}$$

The steps (ii)-(vi) are reiterated until the values of the FS and λ are within set tolerance value. The aforementioned procedure was implemented in MATLAB and converged to the desired accuracy in less than 5 iterations.

3.2 ALO application to slope stability problem

In this application, the algorithm is modelled to determine the optimal FS for trial slip surfaces. The slope is divided into 30 slices having a total of 31 control variables. The variation in x_1 and x_{31} is explained in Sect. 3.1, whereas all $\sigma_1 - \sigma_{29}$ vary from [0,1]. The ALO algorithm is implemented in the following seven steps for slope stability problem:

Step 1 Generate the first array of population search agents and calculate their fitness. The values are defined for $Dim + 1$ variables (31 in this case) that represent coordinates of the trial slip surface as shown in Fig. 4a. The first step involves the initialisation of the algorithm by using a set of random ants roaming and antlions laying traps through the search space.

$$Mat_{Ant} = Mat_{Antlion} = \begin{pmatrix} x_1^1 & \sigma_2^1 & \dots & \dots & \sigma_{29}^1 & x_{31}^1 \\ x_1^2 & \sigma_1^2 & \dots & \dots & \sigma_{29}^2 & x_{31}^2 \\ \vdots & \vdots & \vdots & \vdots & \vdots & \vdots \\ x_1^{NP} & \sigma_1^{NP} & \dots & \dots & \sigma_{29}^{NP} & x_{31}^{NP} \end{pmatrix} \begin{matrix} 1 \\ 2 \\ \vdots \\ NP \end{matrix} \tag{24}$$

Step 2 Sort antlions according to their fitness values in an ascending order, and select the antlion with the optimal fitness as the elite.

Step 3 Pick an antlion for each individual ant via roulette wheel. Antlions having a superior fitness value has higher probability of selection through this process.

- 3.1 Create a random walk (Eq. 7), and normalise it (Eq. 8).
- 3.2 Update the values of $c_i^m(Iter)$ and $d_i^m(Iter)$ (Eqs. 5-6).
- 3.3 Update the position of the ant (Eq. 9).

Step 4 Compute the fitness of all ants and update matrix 1. The fitness is inversely proportional to the FS for the selected control variables and is calculated using Spencer's method explained in Sect. 3. The FS is computed only for matrices of ants (Mat_{OAnt}) and not for antlions when the computation is carried out inside the loop (refer to the flowchart in Fig. 5). Moreover, the elite antlion is always in updated antlion population, which is consistent with other evolutionary techniques that retain the optimal solution.

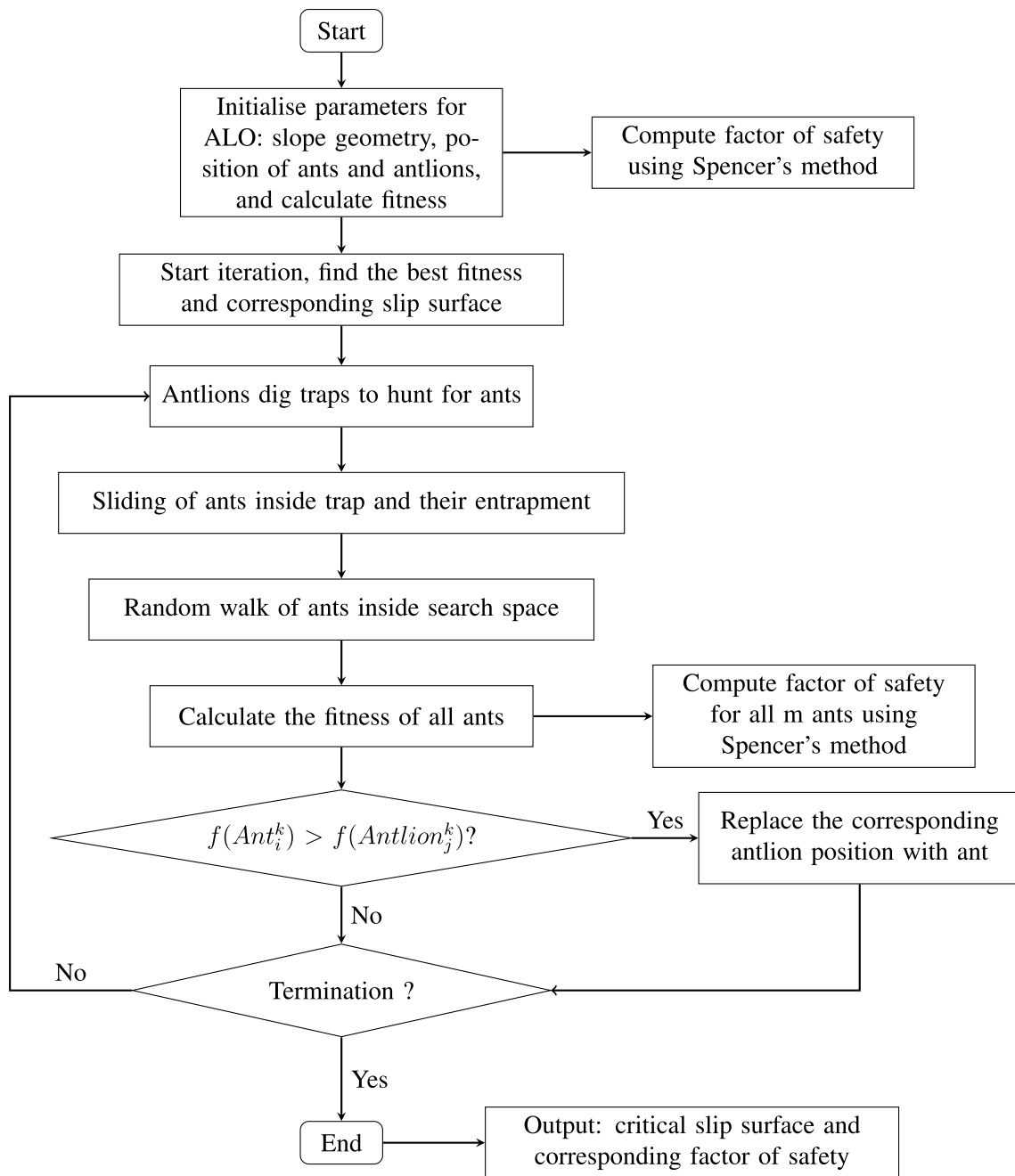


Fig. 5 Flowchart to capture critical slip surface and factor of safety using ALO algorithm for assessing slope stability

Step 5 The antlion exchanges its position with the corresponding ant if the ant is fitter. In this step, the fitness of the selected elite antlion (Eq. 10) improve, thus converging to the optimal solution.

Step 6 Replace elite if $(fit(Ant_i(Iter)) < fit(Antlion_j(Iter)))$.

Step 7 Repeat steps 3–6 until the termination criteria is reached.

The ALO solution comprises numerous generations of the system of variables that are optimised, resulting in the lowest FS. The position of the elitist antlion represents the critical slip surface with a minimum FS. A large value of fitness function is used when the FS violates the search boundary and generates infeasible slip surfaces. Figure 5 the process of the ALO algorithm adopted for slope stability.

Table 2 Upper and lower limits of search variables and number of slices considered for examples 1–4

Example no.	Min x_l	Max x_u	Min x_L	Max x_U	Min σ_u	Max σ_l	No. of Slices N
1.	0	10	20	30	0	1	30
2.	0	60	140	160	0	1	30
3.	10	15	25	34	0	1	30
4.	0	20	40	50	0	1	30

Table 3 Control parameters used in antlion optimiser for factor of safety calculations in examples 1–4

Input variable sets	Example 1	Example 2	Example 3	Example 4
Population (Total number of ants and antlions) (NP)	10–50	10–100	10–100	10–100
Maximum iterations count ($Iter_{max}$)	50	100	100	100
No. of control variables (Dim)	31	31	31	31

4 Numerical experiments

The section analyses four slopes with different complexities, namely a homogeneous slope and three heterogeneous slopes to test the efficacy of ALO approach. The critical slip surface and corresponding FS are determined through the proposed technique and compared with those obtained using other optimisation algorithms. Because the solution obtained in each simulation is different, 10 simulations are used for each numerical experiment to report the mean and standard deviation along with the optimal solution of the numerical experiment. Therefore, for 10 simulations for each numerical experiment, 10 optimal solutions are available for 10 experiments. Among these 10 optimal solution, a solution with the minimum FS is reported as the optimal solution. Moreover, Table 2 reports lower and upper bound limits of control variables. The total vertical slices are fixed to 30 because the computation of the FS of a slope is unaffected by the number of selected slices if the minimum number of slices is set to 30 [87]. Table 3 reports the parameters of ALO. The parameters of the ALO algorithm were chosen from previous studies [78]. Subsequent sections indicate that the critical slip surface gradually approached the optimal slip surface with the progression of the search for these two population sizes.

4.1 Example 1

The homogeneous soil slope considered from Yamagami and Ueta [88] is analysed. The slope is in dry condition with soil parameters reported in Table 4. Figure 6 presents slope geometry. Figure 6 analyses the critical slip surface identified by solving the same benchmark example by using other slope stability optimisation methods

Table 4 Soil parameters for benchmark examples 1–4

Example no.	Layer (s)	Cohesion (c) (kPa)	Friction angle ϕ ($^\circ$)	Unit weight γ (kN/m ³)
1.	1	9.8	10	17.64
2.	1	28.7	20	18.8
	2	0	10	18.8
3.	1	15	20	19.0
	2	17	21	19.0
	3	5	10	19.0
	4	35	28	19.0
4.	1	0	38	19.5
	2	5.3	23	19.5
	3	7.2	20	19.5

and the current technique. In the 10 computations, the calculated minimum and maximum FS are 1.317 and 1.349, respectively. Table 5 presents a comparative summary for two population sizes used with its results verified against different methods. For the homogeneous case, irrespective of the swarm intelligence technique, most of them converge to similar results. The standard deviation in FS is reduced to 0.008 and 0.016 for populations of 50 and 10 from 0.298, respectively, following the results provided by Kahatadeniya et al. [21] by using discrete ant colony optimisation. The current algorithm captures the optimal solution with a fewer number of adjustable parameters to adjust than several prominent algorithms. For the homogeneous case, the analysed algorithms converge to rather similar values, indicating the robustness of calculation.

Figure 7 presents the convergence behaviour of the ALO algorithm. Two simulations are plotted to indicate

Fig. 6 Critical slip surface identified using ALO and other optimisation methods for example 1

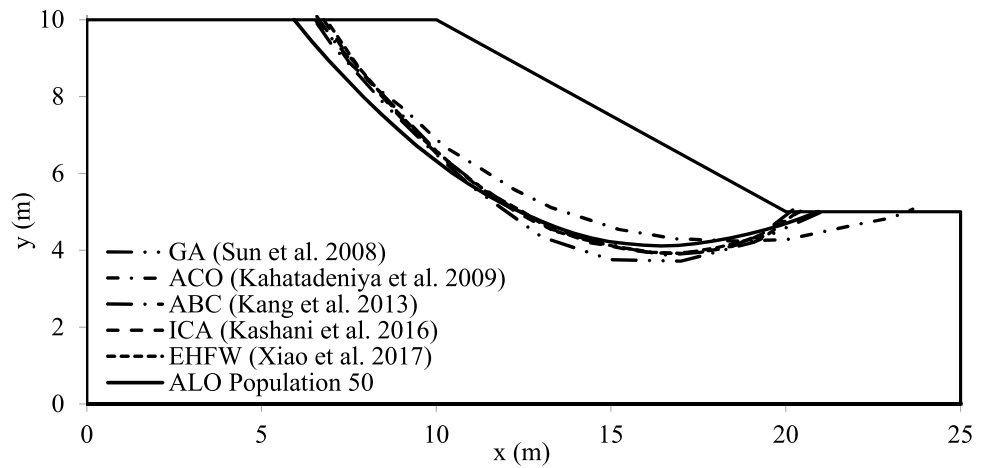


Table 5 Comparison of factor of safety obtained using ALO technique for the standard example 1 with different methods

Source	Method	FS
Yamagami and Ueta [88]	Broyden–Fletcher–Goldfarb–Shanno (BFGS)	1.338
Yamagami and Ueta [88]	Simplex method	1.339–1.348
Cheng et al. [87]	Particle swarm optimisation (PSO)	1.329
Cheng et al. [87]	Modified particle swarm optimisation (MPSO)	1.326
Cheng et al. [20]	Modified harmony search (MHS)	1.322
Jianping et al. [89]	Genetic algorithm (GA) + line	1.324
Jianping et al. [89]	Genetic algorithm (GA) + Spline	1.321
Kahatadeniya et al. [21]	Ant colony optimisation (ACO)	1.311–2.966
Khajehzadeh et al. [25]	Particle swarm optimisation (PSO)	1.321
Khajehzadeh et al. [25]	Modified particle swarm optimisation (MPSO)	1.308
Kang et al. [27]	Artificial bee colony optimisation (ABC)	1.321
Kashani et al. [48]	Imperialistic competitive algorithm (ICA)	1.321
Xiao et al. [43]	Enhanced fireworks algorithm (EFW)	1.322
RS slope [90]	Cuckoo search	1.327
Mishra et al. [50]	Teaching–learning-based optimisation (TLBO)	1.324–1.325
ALO (This study)	Antlion Optimiser (ALO) NP = 10	1.334–1.387
ALO (This study)	Antlion Optimiser (ALO) NP = 50	1.317–1.349

that the fitness of the elitist antlion is updated using the number of iterations and converges to a constant value. The algorithm estimates the FS within desired accuracy,

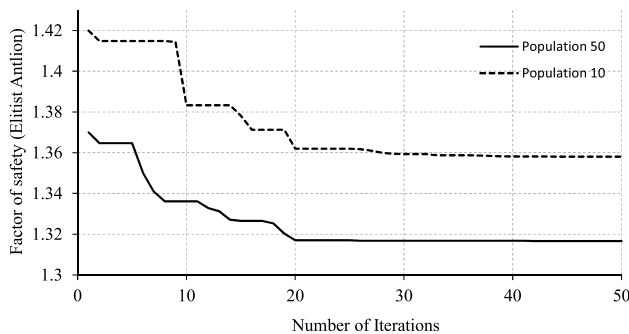


Fig. 7 Factor of safety for fittest antlion versus number of iterations by using populations of 50 and 10 for example 1

even with 10 ants and antlions. The initial estimate is already near to the optimal value and the difference between the optimal values obtained using the populations of 10 and 50 is residual -0.04, which is negligible for engineering applications.

4.2 Example 2

The second illustrative example of the heterogeneous soil profile is referred to from Fredlund and Krahn [91]. Figure 8 presents the geometry of the slope, and soil parameters for three horizontal layers are reported in Table 4. Because of the presence of a thin band of a weak soil layer sandwiched between two strong soil layers, local minima occurs and several traditional methods fail to converge to global minima.

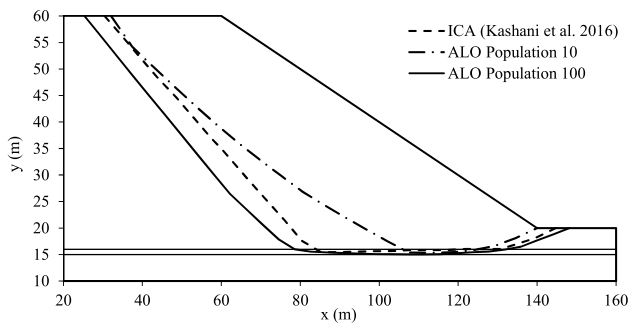


Fig. 8 Critical slip surface identified using ALO and ICA for example 2

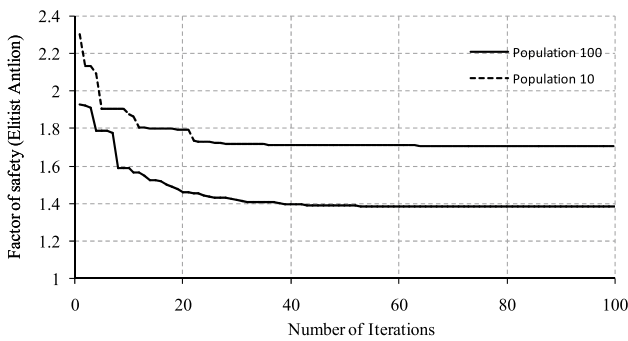


Fig. 9 Factor of safety for fittest antlion vs number of iterations using populations of 100 and 10 for example 2

Figure 8 presents the slip surface for populations of 10 and 100 identified using the current technique. It converges to the optimal solution by increasing the population of both ants and antlions to 100. The current technique can automatically capture the critical failure surface. The obtained minimum FS is 1.366, with a standard deviation of 0.091. The method exhibits consistent results with less standard deviation and a superior estimation of the minimum factor of safety from Zhu et al. [94] and Jongmin et al. [93]. Figure 9 compares the convergence rates of the current technique for population

sizes of 100 and 10 with 100 iterations. The population of ant and antlions for the heterogeneous layer must be approximately 100 to correctly identify global minima. The proposed technique is similar to the recent solution using ICA presented by Kashani et al. [48] and TLBO presented by Mishra et al. [50] with some minor deviations (refer Table 6), which may occur because of the discretisation of the sliding mass into slices and the method used for FS calculations.

4.3 Example 3

The third illustrative example of the heterogeneous soil profile having a weak band of an inclined soil layer is referred to from Zolfaghari et al. [95]. The soil parameters are reported in Table 4. In this case, the weak soil layer is sandwiched between two soil layers. Some optimisation techniques cannot converge to global minima.

The FS and corresponding slip surface have been computed using different optimisation techniques, such as genetic algorithm by Zolfaghari et al. [95]; tabu search, harmony search, particle swarm optimisation, simulated annealing, and ant colony optimisation by Cheng et al. [87]; gravitational search algorithm by Khajehzadeh et al. [30]; artificial bee colony by Kang et al. [27]; cuckoo search, firefly algorithm, and cuckoo search-boundary constraint (CS-BC) by Gandomi et al. [28]; imperialistic competitive algorithm by [48], as presented in Table 7. The solution obtained using the proposed technique is 1.079, which is the optimal solution for this problem. The solution is superior than those obtained using GA, SA, TS, ACO, HS, MHS, PSO, DE, ES FA and MVO. The results are a slightly on higher side when compared with CS, CS-BC, BBO and ICA, reason can be due to the method used to calculate FS as the difference is almost negligible. The standard deviation is obtained with ALO (0.162) is smaller than the one obtained by BBO (0.384) [46] but slightly more than MVO (0.1229) [49].

Figure 10 reports the critical slip surface obtained by solving the benchmark example by using the current

Table 6 Comparison of factor of safety obtained using ALO technique for the standard example 2 with different methods

Source	Method	FS
Fredlund and Krahn [91]	Spencer’s method	1.373
Goh [92]	Genetic algorithm	1.288
Jongmin et al. [93]	Limit equilibrium with the velocity field and plastic zone	1.37
Zhu et al. [94]	A generalised framework of LEM	1.373
Kashani et al. [48]	Imperialistic Competitive Algorithm (ICA)	1.3625
Mishra et al. [50]	Teaching–learning-based optimisation (TLBO)	1.315–1.466
ALO (This study)	Antlion Optimiser (ALO) NP = 10	1.589–2.063
ALO (This study)	Antlion Optimiser (ALO) NP = 100	1.366–1.724

Table 7 Comparison of factor of safety obtained using ALO technique for the standard example 3 with different methods

Source	Method	FS
Zolfaghari et al. [95]	Genetic algorithm (GA)	1.24
Cheng et al. [26]	Simulated annealing (SA)	1.2813
Cheng et al. [87]	Tabu search (TS)	1.4661
Cheng et al. [87]	Ant colony optimisation (ACO)	1.5817
Cheng et al. [87]	Harmony search (HS)	1.2405
Cheng et al. [87]	Modified harmony search (MHS)	1.1315
Cheng et al. [24]	Modified Particle swarm optimisation (MPSO)	1.1289
Cheng et al. [20]	Particle swarm optimisation (PSO)	1.1095
Khajezhadeh et al. [30]	Gravitational search algorithm	1.0785
Kang et al. [27]	Artificial bee colony optimisation (ABC)	1.086
Gandomi et al. [28]	Firefly algorithm (FA)	1.303
Gandomi et al. [28]	Cuckoo search (CS)	1.0635
Gandomi et al. [28]	Cuckoo search—boundary constraint (CS-BC)	1.0502
Gandomi et al. [46]	Biogeography-based optimization (BBO)	1.055
Gandomi et al. [46]	Differential evolution (DE)	1.659
Gandomi et al. [46]	Evolutionary strategy (ES)	1.502
Mishra et al. [49]	Multi verse optimiser (MVO)	1.1447–1.7362
ALO (This study)	Antlion optimiser (ALO) NP = 10	1.253–1.906
ALO (This study)	Antlion optimiser (ALO) NP = 100	1.079–1.633

Fig. 10 Critical slip surface identified using ALO and other optimisation methods for example 3

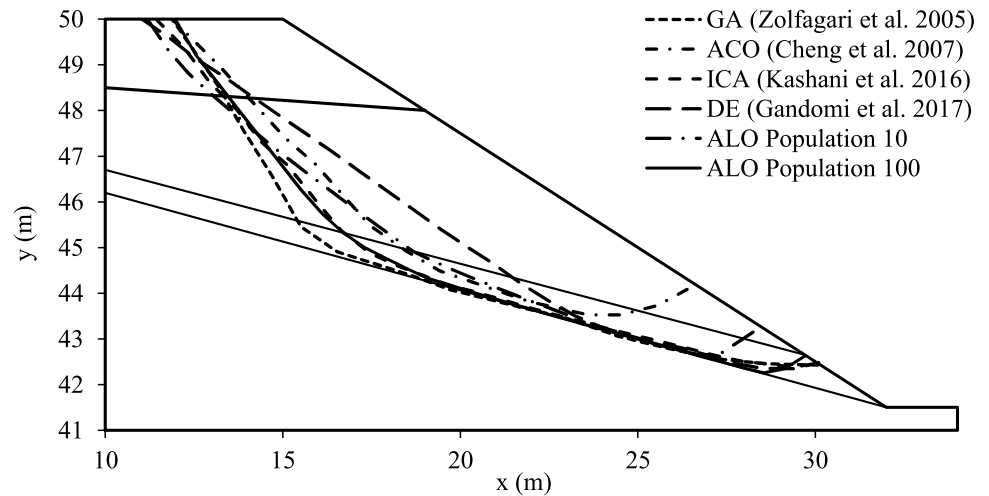


Fig. 11 Factor of safety for fittest antlion vs number of iterations using populations of 100 and 10 for example 4

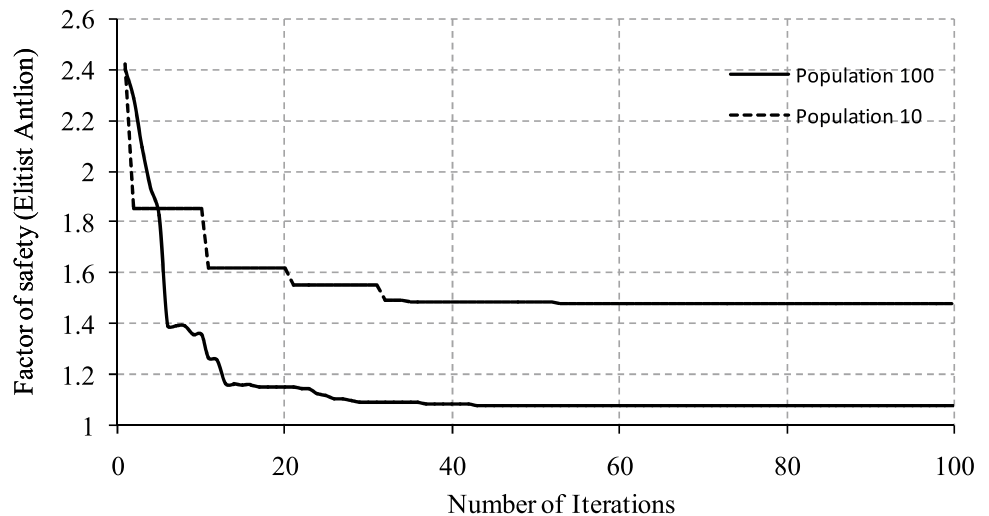
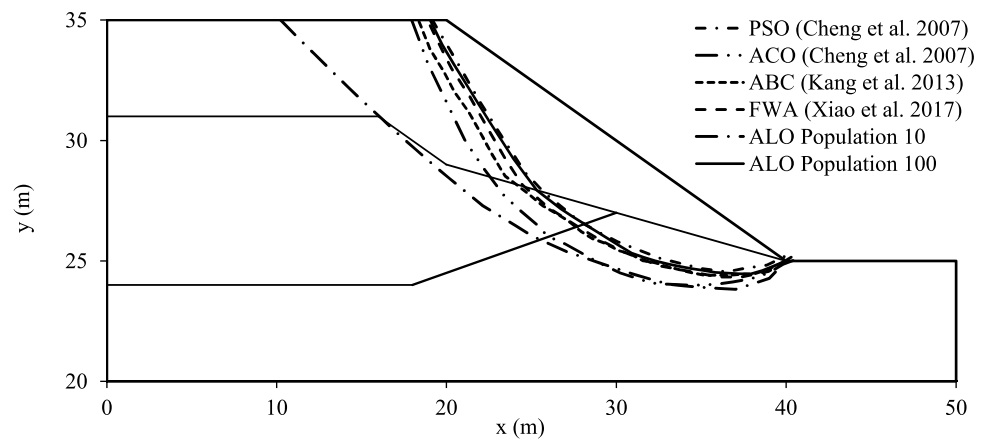


Fig. 12 Critical slip surface identified using ALO and other optimisation methods for example 3



technique for the population sizes of 10 and 100 with 100 iterations. Figure 11 compares the convergence rate of ALO optimiser for population sizes of 10 and 100. For a population size of 100, the ALO more rapidly converges to the optimal solution than with population of 10.

4.4 Example 4

The fourth illustrative example of the heterogeneous soil profile is referred to from the Association for Computer Aided Designs (ACAD) in Australia [96]. The slope has three non-horizontal soil layers whose properties are reported in

Table 4. Figure 12 presents the slip surface for this example captured using the proposed technique. The critical surface obtained using the proposed algorithm is different for population sizes of 10 and 100. Therefore, although the algorithm for both populations converges to similar FS values, the solution is different for each case. Both solutions present similar FS with the different geometry of the critical surface. The problem was previously analysed using GA, leap frog, HS, SA, TS, HS, ABC, ACO, PPACO, TLBO and finite element methods. The minimum factor of safety obtained using ALO is 1.351 with a standard deviation of 0.018, indicating that this method is more efficient than

Table 8 Comparison of factor of safety obtained using ALO technique for the standard example 4 with different methods

Source	Method	FS
Goh [92]	Genetic algorithm (GA)	1.387
Bolton et al. [97]	Leap frog method	1.387
Cheng et al. [87]	Particle swarm optimisation (PSO)	1.359
Cheng et al. [87]	Ant colony optimisation (ACO)	1.3931
Cheng et al. [87]	Simulated annealing (SA)	1.3569
Cheng et al. [87]	Tabu search (TS)	1.3762
Cheng et al. [20]	Harmony search algorithm (HSA)	1.359
Kang et al. [27]	Artificial bee colony optimisation (ABC)	1.343
Gao [45]	Evolutionary programming (EP)	1.358
Gao [45]	Immunised evolutionary programming (IEP)	1.355
Gao [22]	Ant colony optimisation (ACO)	1.353
Gao [23]	Meeting ant colony optimisation (MACO)	1.348
Gao [22]	Premium penalty ant colony optimisation (PPACO)	1.340
Goh [22]	Finite element method	1.426
Gao [98]	Improved black hole algorithm	1.355
Xiao et al. [43]	Enhanced fireworks algorithm (EFW)	1.350
Mishra et al. [50]	Teaching-learning-based optimisation (TLBO)	1.343–1.356
Mishra et al. [49]	Multi verse optimiser (MVO)	1.346–1.376
ALO (This study)	Antlion optimiser (ALO) NP = 10	1.571–1.714
ALO (This study)	Antlion optimiser (ALO) NP = 100	1.351–1.404

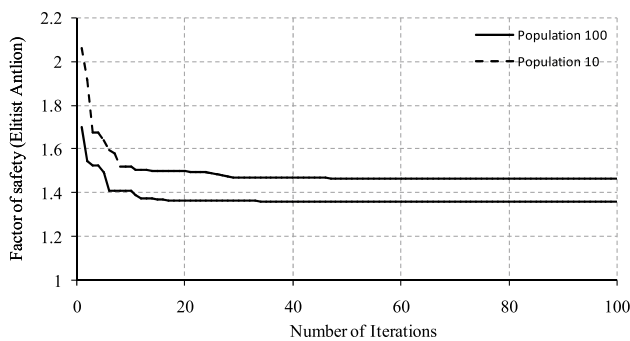


Fig. 13 Factor of safety for fittest antlion versus number of iterations using populations of 100 and 10 for example 4

most methods. However, TLBO presented by Mishra et al. [50] reports the FS between 1.343–1.356 with a standard deviation of 0.003193 making it more efficient in uncertainty reduction.

Table 8 reports the comparison of the FS obtained using the ALO technique for the standard example 4 with different methods previously reported. In this case, the result with FEM (1.426) is sub-optimal solution than result obtained by using the current method (1.351–1.404), which shows that current technique is robust. The ALO is more efficient than other studies because the FS values are similar. Figure 13 compares the convergence rates of ALO optimiser for population sizes of 10 and 100 with 100 iterations. For a population size of 100, ALO more rapidly converges to optimal solution.

Finally, for comparison with other metaheuristic techniques, the number of fitness function evaluations described by NOFs is calculated under the same or replicated experimental conditions. Therefore, comparison is performed by calculating the total fitness evaluations

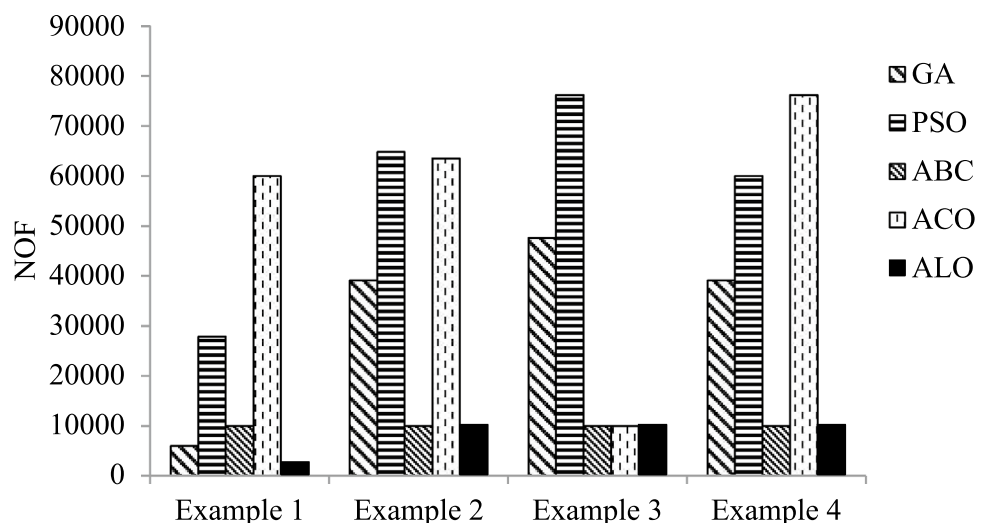
for different population sizes and the number of iterations required for one simulation experiment. All the examples, excluding example 1 that has 2500 fitness function evaluations, used 10,000 fitness function evaluations. Figure 14 shows that the ALO algorithm has fewer fitness function evaluations than GA, PSO, and ACO for all four slopes and similar NOFs than ABC. Although ABC and ALO have the same number of fitness function evaluations, ALO has fewer function fitness function evaluations than GA, PSO, and ACO. In particular, for homogeneous slopes, the performance of ALO is superior in terms of accuracy and NOFs consumed (refer to a small bump shown in Fig. 14 for example 1 reporting 2500 NOFs) and for heterogeneous slopes it can locate the critical slip surface with success consuming 10,000 NOFs. The proposed approach yields accurate results when maintaining the number of fitness function evaluations at minimum in comparison with other optimisation techniques.

The computational time using population of 10 and 100 is further assessed based on a Intel(R) Core(TM) i5-7500 CPU 3.40 GHz processors with 4.00 GB of installed memory (RAM). The average computing time is about 5 s for the population of 10 and 30 s for the population of 100 with 100 iterations for homogeneous slope. For heterogeneous slope profiles, the time increases to 7 s and 40 s for 100 iterations respectively.

5 Conclusions

A comprehensive state-of-the-art review of the slope stability using swarm intelligence techniques is presented in this paper. Additionally, from the state-of-the-art reviews that the applications of swarm intelligence

Fig. 14 Comparison of total number of function evaluations (NOFs) of antlion optimisation with other optimisation techniques



techniques are discussed with emphasis on parameter tuning, which is drawback of several methods. For the comparison purposes, the ALO algorithm is successfully adapted to solve the slope stability problem for four different examples with different complexities. On the basis of the four case studies analysed, the algorithm can reach satisfactory results, and therefore, it is robust and in line with other evolutionary algorithms already applied to the same cases in the past.

The fitness function evaluations for ALO is less than or similar to other optimisation approaches. It has been observed from the present study that the fitness function zeroed in on the global minima and unlike some bio-inspired algorithms, it requires fewer parameters to adjust. One of the primary advantages of ALO over other prominent algorithms is that it has a fairly simple mathematical structure and requires only two parameters, namely the number of search agents and iterations, to be adjusted. ALO used roulette wheel selection and elite antlion bias to select control parameter as compared with ACO, which is primarily driven by pheromone level updates. Therefore, compared with ACO, the proposed method has wider search capabilities and can explore solutions by using more combinations of control variables. This mechanism assists ALO in coming out of local minimum more easily than its counterpart ACO. The results from numerical experiments demonstrate that solutions realised using ALO and TLBO have less standard deviation than those obtained using the existing optimisation methods. Therefore, the reviews of current study can be applied by various researchers and field practitioners for choosing a particular metaheuristic technique for their application.

The proposed analysis neglects the effect of pore water pressure as all the case study examples are analysed based on dry conditions (i.e. zero pore water pressure). Furthermore, stability evaluation of a slope in case of a seismic condition is not addressed in the paper. In the future works, effect of pore water pressure and earthquake forces can be taken into account as they have a direct effect on computing the safety factor of the slopes. Furthermore, effect of the number of vertical slices (current study 30 are used) on slopes of varying complexity and their effect on factor of safety can also be taken into consideration.

Compliance with ethical standards

Conflict of interest The authors declare that they have no conflict of interest.

References

1. Evans SG, Roberts NJ, Ischuk A, Delaney KB, Morozova GS, Tutubalina O (2009) Landslides triggered by the 1949 Khait earthquake, Tajikistan, and associated loss of life. *Eng Geol* 109(3–4):195–212
2. Ahmed B (2015) Landslide susceptibility modelling applying user-defined weighting and data-driven statistical techniques in Cox's Bazar Municipality, Bangladesh. *Nat Hazards* 79(3):707–737
3. Ubydul H, Philipp B et al (2016) Fatal landslides in Europe. *Landslides* 13(6):1545–1554
4. Duric D, Mladenovic A, Pesic-Georgiadis M et al (2017) Using multiresolution and multitemporal satellite data for post-disaster landslide inventory in the Republic of Serbia. *Landslides* 14(4):1467–1482
5. Cheng YM, Lansivaara T, Wei WB (2007) Two-dimensional slope stability analysis by limit equilibrium and strength reduction methods. *Comput Geotech* 34(3):137–150
6. Bishop AW (1955) The use of the slip circle in the stability analysis of slopes. *Géotechnique* 5:7–17
7. Janbu N (1973) Slope stability computations. In: Hirschfeld RC, Poulos SJ (eds) *Embankment dam engineering*. Wiley, New York
8. Morgenstern NR, Price VE (1965) The analysis of the stability of general slip surfaces. *Géotechnique* 15(1):79–93
9. Viggiani C (1981) Ultimate lateral load on piles used to stabilize landslides. In: *Proceedings of the 10th international conference on soil mechanics and foundation engineering*, Balkema, Rotterdam, vol. 3, pp 555–560
10. Zhu DY, Lee CF (2002) Explicit limit equilibrium solution for slope stability. *Int J Numer Anal Methods Geomech* 26:1573–1590
11. Li X, Su L, He S, Xu J (2016) Limit equilibrium analysis of seismic stability of slopes reinforced with a row of piles. *Int J Numer Anal Methods Geomech* 40:1241–1250
12. Chen ZY, Shao CM (1988) Evaluation of minimum factor of safety in slope stability analysis. *Can Geotech J* 25(4):735–748
13. *Fast Lagrangian Analysis of Continua (FLAC) (2005) version 5.0*. ITASCA Consulting Group, Inc., Minneapolis
14. Duncan JM (1996) State of the art: limit equilibrium and finite element analysis of slopes. *J Geotech Eng* 122(7):577–596
15. Griffiths DV, Lane PA (1999) Slope stability analysis by finite elements. *Géotechnique* 49(3):387–403
16. Zheng H, Sun G, Liu D (2009) A practical procedure for searching critical slip surfaces of slopes based on the strength reduction technique. *Comput Geotech* 36(1):1–5
17. Nasvi MCM, Krishnya S (2019) Stability analysis of Colombo-Katunayake expressway (CKE) using finite element and limit equilibrium methods. *Indian Geotech J*. <https://doi.org/10.1007/s40098-019-00357-7>
18. Mahdiyar A, Hasanipanah M, Armaghani DJ et al (2017) A Monte Carlo technique in safety assessment of slope under seismic condition. *Eng Comput* 33(4):807–817
19. Mojtahedi SFF, Tabatabaee S, Ghorogi M et al (2018) A novel probabilistic simulation approach for forecasting the safety factor of slopes: a case study. *Eng Comput* 35:1–10
20. Cheng YM, Li L, Lansivaara T, Chi SC, Sun YJ (2008) An improved harmony search minimization algorithm using different slip surface generation methods for slope stability analysis. *Eng Optim* 40(2):95–115
21. Kahatadeniya KS, Nanakorn P, Neaupane KM (2009) Determination of the critical failure surface for slope stability analysing ant colony optimization. *Eng Geol* 108:133–141

22. Gao W (2016) Premium-penalty ant colony optimization and its application in slope stability analysis. *Appl Soft Comput* 43:480–488
23. Gao W (2016) Determination of non-circular critical slip surface in slope stability analysis by meeting ant colony optimization. *J Comput Civil Eng ASCE* 30(2):06015001
24. Cheng YM, Li L, Chi S, Wei WB (2007) Particle swarm optimization algorithm for the location of the critical non-circular failure surface in two-dimensional slope stability analysis. *Comput Geotech* 34(2):92–103
25. Khajehzadeh M, Taha MR, El-Shafie A, Eslami M (2012) Locating the general failure surface of earth slope using particle swarm optimization. *Civil Eng Environ Syst* 29(1):41–57
26. Cheng YM (2007) Global optimization analysis of slope stability by simulated annealing with dynamic bounds and dirac function. *Eng Optim* 39(1):17–32
27. Kang F, Li J, Ma Z (2013) An artificial bee colony algorithm for locating the critical slip surface in slope stability analysis. *Eng Optim* 45(2):207–223
28. Gandomi AH, Kashani AR, Mousavi M, Jalalvandi M (2014) Slope stability analyzing using recent swarm intelligence techniques. *Int J Numer Anal Methods Geomech* 39(3):295–309
29. Cheng YM, Liang L, Chi SC, Wei WB (2008) Determination of the critical slip surface using artificial fish swarms algorithm. *J Geotech Geoenviron Eng* 134(2):244–251
30. Khajehzadeh M, Taha MR, El-shafie A, Eslami M (2011) Search for critical failure surface in slope stability analysis by gravitational search algorithm. *Int J Phys Sci* 6(21):5012–5021
31. Khajehzadeh M, Taha MR, El-Shafie A, Eslami M (2012) A modified gravitational search algorithm for slope stability analysis. *Eng Appl Artif Intell* 25(8):1589–1597
32. Singh J, Banka H, Verma AK (2018) Analysis of slope stability and detection of critical failure surface using gravitational search algorithm. In: 4th International conference on recent advances in information technology (RAIT), Dhanbad, pp 1–6
33. Saha A (2013) Big-bang big-crunch optimization in locating the critical surface in slope-stability. In: Proceedings of Indian geotechnical conference, Roorkee, Paper No. 82
34. Zhao H, Yin S, Ru Z (2012) Relevance vector machine applied to slope stability analysis. *Int J Numer Anal Methods Geomech* 36(5):643–652
35. Hu C, Jimenez R, Li SC, Li LP (2013) Determination of critical slip surfaces using mutative scale chaos optimization. *J Comput Civil Eng* 29(5):04014067
36. Tongchun H (2012) Stability calculation of slope by a tabu search method. In: Fourth joint international symposium on information technology in civil engineering, towards a vision for information technology in civil engineering
37. Sengupta A, Upadhyay A (2009) Locating the critical failure surface in a slope stability analysis by genetic algorithm. *Appl Soft Comput* 9(1):387–392
38. Pina RJ, Jimenez R (2015) A genetic algorithm for slope stability analyses with concave slip surfaces using custom operators. *Eng Optim* 47(4):453–472
39. Manouchehran A, Gholamnejad J, Sharifzadeh M (2014) Development of a model for analysis of slope stability for circular mode failure using genetic algorithm. *Environ Earth Sci* 71(3):1267–1277
40. Zhu JF, Chen CJ (2014) Search for circular and noncircular critical slip surfaces in slope stability analysis by hybrid genetic algorithm. *J Central South Univ* 21(1):387–397
41. McCombie P, Wilkinson P (2002) The use of the simple genetic algorithm in finding the critical factor of safety in slope stability analysis. *Comput Geotech* 29(8):699–714
42. Goh ATC (2000) Search for critical slip circle using genetic algorithms. *Civil Eng Environ Syst* 17(3):181–211
43. Xiao Z, Tian B, Lu X (2017) Locating the critical slip surface in a slope stability analysis by enhanced fireworks algorithm. *Cluster Comput* 22:1–11
44. Gao W, Wang X, Dai S, Chen D (2016) Study on stability of high embankment slope based on black hole algorithm. *Environ Earth Sci* 75(20):1381
45. Gao W (2015) Slope stability analysis based on immunised evolutionary programming. *Environ Earth Sci* 74(4):3357–3369
46. Gandomi AH, Kashani AR, Mousavi M, Jalalvandi M (2017) Slope stability analysis using evolutionary optimization techniques. *Int J Numer Anal Methods Geomech* 41(2):251–264
47. Singh J, Banka H, Verma AK (2018) A BBO-based algorithm for slope stability analysis by locating critical failure surface. *Neural Comput Appl* 31:1–18
48. Kashani AR, Gandomi AH, Mousavi M (2016) Imperialistic competitive algorithm: a metaheuristic algorithm for locating the critical slip surface in 2-dimensional soil slopes. *Geosci Front* 7(1):83–89
49. Mishra M, Gunturi RV, Maity D (2019a) Multiverse optimisation algorithm for capturing the critical slip surface in slope stability analysis. *Geotech Geol Eng*. <https://doi.org/10.1007/s10706-019-01037-2>
50. Mishra M, Gunturi RV, Maity D (2019b) Teaching–learning-based optimisation algorithm and its application in capturing critical slip surface in slope stability analysis. *Soft Comput*. <https://doi.org/10.1007/s00500-019-04075-3>
51. Gandomi AH, Kashani AR, Mousavi M (2015) Boundary constraint handling affection on slope stability analysis. In: Lagaros N, Papadrakakis M (eds) Engineering and applied sciences optimization. Springer, Basel, pp 341–358
52. Liang L, Xue-Song C (2011) An improved particle swarm optimization algorithm with harmony strategy for the location of critical slip surface of slopes. *China Ocean Eng* 25(2):357–364
53. Dong H, Jian-Ping Q (2010) Hybrid of ant colony algorithm and simulated annealing algorithm and its application to the slope stability analysis. In: 2010 Sixth international conference on natural computation vol. 6, pp 3329–3333
54. Raihan TM, Mohammad K, Mahdiyeh E (2011) A new hybrid algorithm for global optimization and slope stability evaluation. *J Central South Univ* 20(11):3265–3273
55. Sakellariou MG, Ferentinou MD (2005) A study of slope stability prediction using neural networks. *Geotech Geol Eng* 23(4):419–445
56. Kaunda RB, Chase RB, Kehew AE, Kaugars K, Selegean JP (2010) Neural network modeling applications in active slope stability problems. *Environ Earth Sci* 60(7):1545–1558
57. Choobbasti AJ, Farrokhzad F, Barari A (2009) Prediction of slope stability using artificial neural network (case study: Noabad, Mazandaran, Iran). *Arab J Geosci* 2(4):311–319
58. Erzini Y, Cetin T (2012) The use of neural networks for the prediction of the critical factor of safety of an artificial slope subjected to earthquake forces. *Scientia Iranica* 19(2):188–194
59. Chok YH, Jaks MB, Kaggwa WS, Griffiths DV, Fenton GA (2016) Neural network prediction of the reliability of heterogeneous cohesive slopes. *Int J Numer Anal Methods Geomech* 40:1556–1569
60. Hoang N, Pham A (2016) Hybrid artificial intelligence approach based on metaheuristic and machine learning for slope stability assessment: a multinational data analysis. *Expert Syst Appl* 46:60–68
61. Kang F, Li J, Xu Q (2017) System reliability analysis of slopes using multilayer perceptron and radial basis function networks. *Int J Numer Anal Methods Geomech* 41:1962–1978
62. Das SK, Biswal RK, Sivakugan N, Das B (2011) Classification of slopes and prediction of factor of safety using differential evolution neural networks. *Environ Earth Sci* 64(1):201–210

63. Mahdevari S, Shahriar K, Sharifzadeh M et al (2017) Stability prediction of gate roadways in longwall mining using artificial neural networks. *Neural Comput Appl* 28(11):3537–3555
64. Gordan B, Jahed-Armaghani D, Hajihassani M, Monjezi M (2016) Prediction of seismic slope stability through combination of particle swarm optimization and neural network. *Eng Comput* 32:85–97
65. Zhang RH, Goh ATC, Zhang WG (2019) System reliability assessment on deep braced excavation adjacent to an existing upper slope in mountainous terrain: a case study. *SN. Appl Sci* 1:876
66. Ye S, Fang G, Ma X (2019) Reliability analysis of grillage flexible slope supporting structure with anchors considering fuzzy transitional interval and fuzzy randomness of soil parameters. *Arab J Sci Eng* 44(10):8849–8857. <https://doi.org/10.1007/s13369-019-03912-9>
67. Qi C, Tang X (2018) A hybrid ensemble method for improved prediction of slope stability. *Int J Numer Anal Methods Geomech* 42:1823–1839
68. Wolpert DH, Macready WG (1997) No free lunch theorems for optimization. *IEEE Trans Evol Comput* 1(1):67–82
69. Bagheri Sereshki A, Derakhshani A (2019) Optimizing the mechanical stabilization of earth walls with metal strips: applications of swarm algorithms. *Arab J Sci Eng* 44(5):4653. <https://doi.org/10.1007/s13369-018-3492-8>
70. Kaveh A, Behnam AF (2013) Charged system search algorithm for the optimum cost design of reinforced concrete cantilever retaining walls. *Arab J Sci Eng* 38(3):563–570. <https://doi.org/10.1007/s13369-012-0332-0>
71. Mirjalili S (2015) The ant lion optimizer. *Adv Eng Softw* 83:80–98
72. Ali ES, Abd-Elazim SM, Abdelaziz AY (2016) Ant lion optimization algorithm for renewable distributed generations. *Energy* 116(1):445–458
73. Zawbaa HM, Emery E, Parv B (2015) Feature selection based on antlion optimization algorithm. *Third World Conf Complex Syst* 2015:1–7
74. Kamboj VK, Bhadoria A, Bath SK (2017) Solution of non-convex economic load dispatch problem for small-scale power systems using ant lion optimizer. *Neural Comput Appl* 28(8):2181–2192
75. Kilic H, Yuzgec U, Karakuzu C (2018) A novel improved antlion optimizer algorithm and its comparative performance. *Neural Comput Appl*. <https://doi.org/10.1007/s00521-018-3871-9>
76. Mani M, Bozorg-Haddad O, Chu X (2018) Ant lion optimizer (ALO) algorithm. In: Bozorg-Haddad O (ed) *Advanced optimization by nature-inspired algorithms. Studies in computational intelligence*, vol 720. Springer, Singapore, pp 105–116. https://doi.org/10.1007/978-981-10-5221-7_11
77. Kaveh M, Amiri Chayjan R, Taghinezhad E, Abbaspour Gilandeh Y, Younesi A, Rasooli Sharabiani V (2019) Modeling of thermodynamic properties of carrot product using ALO, GWO, and WOA algorithms under multi-stage semi-industrial continuous belt dryer. *Eng Comput* 35(3):1045–1058. <https://doi.org/10.1007/s00366-018-0650-2>
78. Mishra M, Barman SK, Maity D, Maiti DK (2019c) Ant lion optimization algorithm for structural damage detection using vibration data. *J Civil Struct Health Monit* 9(1):117–136. <https://doi.org/10.1007/s13349-018-0318-z>
79. Subhashini KR, Satapathy JK (2017) Development of an enhanced ant lion optimization algorithm and its application in antenna array synthesis. *Appl Soft Comput* 59:153–173
80. Crepinšek M, Liu SH, Mernik L, Mernik M (2016) Is a comparison of results? Meaningful for the inexact replications of computational experiments? *Soft Comput* 20(1):223–235
81. Mernik M, Liu SH, Karaboga D, Crepinšek M (2015) On clarifying misconceptions when comparing variants of the artificial bee colony algorithm by offering a new implementation. *Inf Sci* 291:115–127
82. MATLAB (2010) version 7.10.0 (R2010a). The MathWorks Inc., Natick, Massachusetts
83. Cheng YM (2003) Locations of critical failure surface and some further studies on slope stability analysis. *Comput Geotech* 30:255–267
84. Lo S-CR, Xu D-W (1992) A strain based design method for the collapse limit state of reinforced soil walls and slopes. *Can Geotech J* 29(8):832–842
85. Zhu DY, Lee CF, Qian QH, Zou ZS, Sun F (2005) A concise algorithm for computing the factor of safety using the Morgenstern-Price method. *Can Geotech J* 42(1):272–278
86. Spencer E (1967) A method of analysis of the stability of embankments assuming parallel inter-slice forces. *Géotechnique* 17:11–26
87. Cheng YM, Li L, Chi SC (2007) Performance studies on six heuristic global optimization methods in the location of critical failure surface. *Comput Geotech* 34:462–484
88. Yamagami T, Ueta Y (1988) Search for noncircular slip surfaces by the morgenstern-price method. In: *The sixth international conference numerical methods in geomechanics*, pp 1335–1340
89. Jianping S, Li J, Liu Q (2008) Search for critical slip surface in slope stability analysis by spline-based GA method. *J Geotech Geoenviron Eng* 134(2):252–256
90. Rocscience (2002) *Slide, a synopsis of slope stability analysis. Technical report*, Rocscience
91. Fredlund DG, Krahn J (1977) Comparison of slope stability methods of analysis. *Can Geotech J* 13(3):429–439
92. Goh ATC (1999) Genetic algorithm search for critical slip surface in multiple-wedge stability analysis. *Can Geotech J* 36:382–391
93. Jongmin K, Salgado R, Juhnwan L (2002) Stability analysis of complex soil slopes using limit analysis. *J Geotech Geoenviron Eng* 7:546–557
94. Zhu DY, Lee CF, Jiang HD (2003) Generalised framework of limit equilibrium methods for slope stability analysis. *Géotechnique* 53(4):377–395
95. Zolfaghari AR, Heath AC, McCombie PF (2005) Simple genetic algorithm search for critical non-circular failure surface in slope stability analysis. *Comput Geotech* 32(3):139–152
96. Donald IB (1989) *Soil slope stability programs review. Association for computer aided design, review (Australia)*
97. Bolton HPJ, Heymann G, Groenwold AA (2003) Global search for critical failure surface in slope stability analysis. *Eng Optim* 35(1):51–65
98. Gao W (2017) Investigating the critical slip surface of soil slope based on an improved black hole algorithm. *Soils Found* 57:988–1001

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