

# Estimation of Surface Runoff from Semi-arid Ungauged Agricultural Watershed Using SCS-CN Method and Earth Observation Data Sets

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**Abstract** In the present study, Soil Conservation Service - Curve Number (SCS-CN) method, Earth Observation (EO) data sets and Geographic Information System (GIS) have been used in order to estimate the surface runoff from Jhagrabaria an agricultural watershed of Allahabad district, Uttar Pradesh (India). LANDSAT-7ETM+, NOAA data and hydrologic soil groups have been used to prepare land use/land cover, rainfall and soil map. The traditional SCS-CN method for calculating the composite CN is very tedious and time demanding process of the hydrologic modeling. Therefore, GIS is now being used in combination with the SCS-CN method. The outcome of work showed 79.35 (CN<sub>I</sub>) of normal condition, of dry condition 61.76 (CN<sub>I</sub>) and of wet condition 89.84 (CN<sub>III</sub>), respectively. This investigation outline that ungauged watershed exhibits an annual average of 14 years runoff volume as  $3.58 \times 10^6 \text{ m}^3$  from an average annual rainfall of 14 years 110.77 cm and the average annual surface runoff of 14 years was 23.83 cm and annual average runoff coefficient of 14 years was 0.22. The correlation analysis suggests that the strong correlation as  $R^2$  (0.91) was observed between satellite drive rainfall and runoff from SCS-CN method. The developed rainfall–runoff model for the region will be useful to understand the watershed and its runoff flow characteristics.

**Keywords** SCS-CN · Hydrological model · Rainfall · Runoff · Watershed

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## Introduction

Often, the hydrological studies needs records of rainfall and runoff to understand the management of water. Watershed characteristics which may be most readily compared to estimate the volume of runoff that will result of rainfall, soil type and land use/land cover (LULC).

The application of conceptual hydrological models in ungauged watersheds or watersheds with limited data to generate runoff records for planning and design purposes is intriguing [8, 17]. In such applications, the hydrological models are calibrated to gauged watersheds of a homogeneous region, and regional equations helps in explaining the variations of the model parameters with physiographic factors. However, the uncertainty of calibrated model parameters is high enough to simulate the hydrologic response of ungauged watersheds with reduced efficiency [8].

Earth Observation (EO) data and Geographical Information System (GIS) together have played a critical role in watershed modeling [19] in general and rainfall-runoff modeling [6, 12, 32] in particular. Many studies have proved the use of EO data namely groundwater [27, 46, 48, 51, 52], river water quality [55], coastal water [22], lake and wetlands [5, 47, 53, 59, 60], land use/land cover mapping [45, 46, 48, 50, 51], land use change trajectories [56], land use/land cover modeling [28, 49], urban land use dynamics [4], hydrological modeling [31], forest mapping [44], cyclone tracking [16], soil characterization [37], climate change [54], slope estimation [57], landscape ecology [47, 53], ocean studies [35, 36] and watershed management [67], watershed prioritization [68]. GIS processing has become a critical step in hydrologic modeling since it contributes to generate model parameters in a spatially distributed manner. It has involved in data storage, retrieval, map overlay, spatial map analysis etc. and to derive hydrologic parameters from soil, land cover, and rainfall etc.

The SCS-CN method has been widely used since the availability of tabulated curve number (CN) values for rainfall-runoff model [7, 34, 66]. Moore and Clarke [26] showed that a variety of distributions can be easily incorporated into this type of model structure and they derive analytical equations for the responses of different distributions. Hosking and Clarke [14] has extended Moore and Clarke work, to show how the model can be used to derive a relationship between the frequencies of storm rainfall and magnitude of peak flows in an analytical form. Moore [25] examines the case where the stores/lose of water to deep drainage and evapotranspiration. Recent work at the UK Institute of Hydrology has shown the model applications for long runs to derive flood frequencies [11, 23], and also in a more distributed application with radar rainfall and snowmelt inputs for flood forecasting.

Ahmad and Verma [2] have successfully integrated SCS-CN method with EO data into GIS platform and estimated runoff for Kharun River sub-basin. Köylü and Geymen [21] applied SCS-CN method to calculate the runoff in the catchment basin of the Yamula Dam (Kızılırmak River of Turkey) and found SCS-CN method is sensitive to LULC change and change of CN (change of the runoff values). Chavda et al. [9] implemented SCS-CN method for estimation of surface runoff and water availability for two sites in Junagadh, Gujarat and suggested surface water harvesting structures. Topno et al. [63], also computed annual runoff depth for ungauged catchment area (Vindhychal region) and revealed that the SCS-CN method can be used to estimate surface runoff depth when adequate hydrological information is not available. Thomas [61] implemented SCS-CN method over Olifants River catchment (South Africa) area with the help of EO data and GIS and generated spatially distributed infiltration maps for the catchment. Muthu and Santhi [30] applied SCS-CN method over Indian region (Thiruporur Block, Kancheepuram district) using seven rain gauge station data and generated seasonal variation of rainfall runoff of the study area. Ningaraju et al. [33] also used this model over ungauged (Kharadya milli watershed, Mandya district of Karnataka) watershed and found a good co-relationship ( $R^2 = 0.94$ ) between estimated runoff and rainfall for 11 years of datasets. Aldoma and Mohamed [3]; simulated rainfall runoff process for Khartoum state (Sudan) using SCS-CN method and EO data as input of model and found SCS-CN method is capable for predicting runoff. Tirkey et al. [62], estimated runoff (for Jharkhand, India) using SCS-CN method and compared with observed runoff and found a strong correlation between rainfall and runoff as well as between observed runoff and estimated runoff with high accuracy of runoff estimation by SCS-CN method. Fan et al. [13] have developed a simulation model based on the SCS-CN method to analyze the rainfall-runoff relationship in Guangzhou (in southern China). Xianzhao and Jiazhu [66], estimated runoff during flood period from small watershed in Loess Plateau of China using SCS-CN method and found

SCS-CN method is legitimate and can be successfully used to simulate the runoff generation and the runoff process of typical small watershed. Khosravi et al. [20], accept SCS-CN method as better (due to easiness of model) than other empirical models and compared nine empirical models (which were independent from LULC and soil data) with SCS-CN method base runoff.

The aims of the work were as follows: (i) to calculate daily runoff (mm) using CN method and (ii) to find out optimal empirical mathematical model (EMM) with respect to SCS-CN, EO data sets and GIS based model for generating annual runoff (cm) in ungauged Jhagrabaria an agricultural watershed of Allahabad district, U.P. (India).

## Materials and Methods

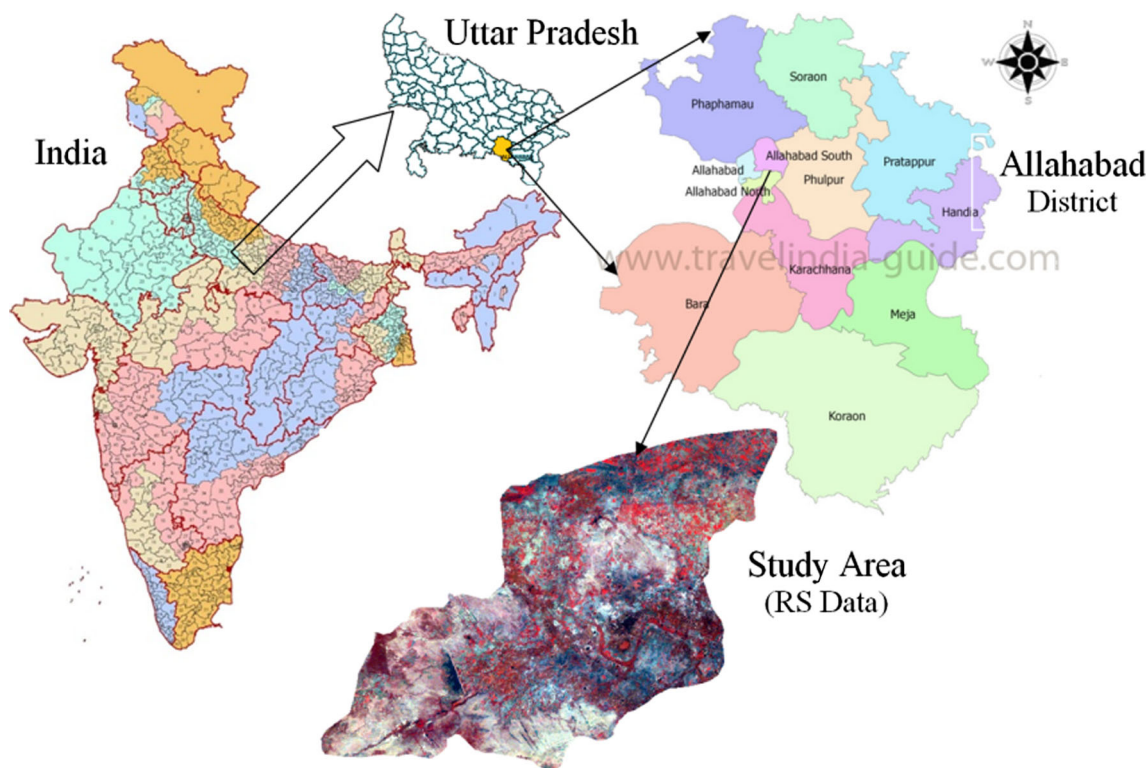
### Study Area

The Jhagrabaria watershed is lies in latitudes  $25^{\circ}12''$ - $25^{\circ}20''$  N and longitudes  $81^{\circ}33''$ - $81^{\circ}44''$  E with an area of  $150.06 \text{ km}^2$  (Fig. 1) in district Allahabad of state Uttar Pradesh, India. The area is dominated by sandstone and shale of Upper Vindhayan formations. The slope of area shows a nearly flat to a gentle undulating topography with occasionally small hillocks. The elevation ranges from 90–180 m above mean sea level [43].

### Data Sets Used and Methodology

The comparative specification of LANDSAT TM and ETM+ sensors are given in Table 1. LANDSAT-7ETM+ of month June, 2006 was downloaded on October 2, 2012, and it was distorted with scene line error and which was rectified using gap filling program in ENVI 4.8 software. Generally, the atmospheric effect is not corrected by data providers; it should be done by the users as a pre-processing task [29]. It was geometrically corrected using Survey of India (SOI) topographic maps (1:50,000, 63G/11 and 63G/12). The ground-truth work (150 ground-truth data/reference data) was performed in the month of August 2012. LULC types namely fallow land, barren land, vegetation and water bodies/wetlands were identified and their co-ordinates were logged with a hand held Global Positioning System (GPS) device (Garmin (eTrexH), with  $\pm 15 \text{ m}$  horizontal locational accuracy. These data were used in supervised classification method [43]. For 'salt and pepper' noise correction from classified image, a majority filter with a  $3 \times 3$  window size was used.

The soil map was obtained from National Bureau of Soil Survey and Land Use Planning (NBSS and LUP), Nagpur, Maharashtra, India. The map was scanned and then geo-referenced with help of SOI topographical maps. Further, the geo-referenced soil map was digitized and soil attributes were assigned.



**Fig. 1** Location map of the study area (Jhagrabaria watershed) in the state of Uttar Pradesh (India)

The rainfall data of entire duration and area was derived from daily rainfall satellite data as provided by the National Oceanic and Atmospheric Administration (NOAA) were downloaded from web-portal. The need of satellite estimated rainfall arises mainly due scare to rain gauges [40]. The daily rainfall estimate is provided for the South Asia (70°–110° E; 5°–35° N) beginning from May 01, 2001. The product is updated three times daily at around 9.00 a.m., 1.00 p.m. and 9.00 p.m. eastern local time and covers a 24-h period of accumulated rainfall. Resolution of rainfall estimates is 0.1°×0.1° and inputs include GTS station data, as well as GOES GPI infrared cloud top temperature fields derived from Meteosat

and polar orbiting satellite rainfall estimate data from SSM/I on board defense meteorological satellite program and AMSU-B on board NOAA15, 16 and 17 [40–42] (Fig. 2).

*Description of SCS-CN method*

In the early 1950s, the United States Department of Agriculture (USDA), NRCS (later named as Soil Conservation Service, SCS) developed a method to estimate the volume of direct runoff from rainfall. It is often referred as the curve number (CN) method, was empirically developed for small agricultural

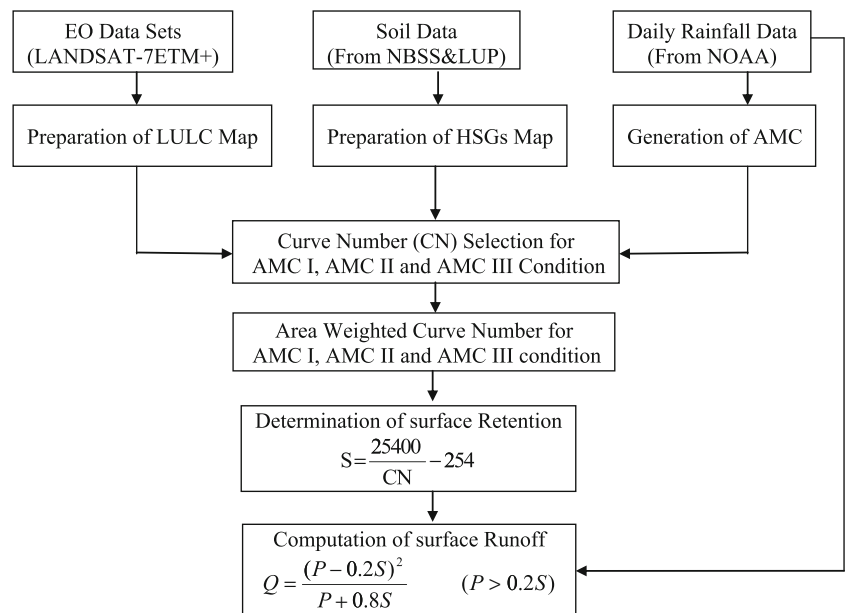
**Table 1** Comparative specification of LANDSAT TM and ETM+ sensors

Bands	Wavelength (μm)		Resolution (m)	
	TM	ETM+	TM	ETM+
Band 1—blue	0.45–0.52	0.45–0.52	30	30
Band 2—green	0.52–0.60	0.52–0.60	30	30
Band 3—red	0.63–0.69	0.63–0.69	30	30
Band 4—near infrared (NIR)	0.76–0.90	0.77–0.90	30	30
Band 5—shortwave infrared (SWIR)1	1.55–1.75	1.55–1.75	30	30
Band 6—thermal	10.40–12.50	10.40–12.50	120 <sup>a</sup> (30)	60 <sup>b</sup> (30)
Band 7—shortwave infrared (SWIR) 2	2.08–2.35	2.09–2.35	30	30
Band 8—panchromatic	—	0.52–0.90	—	15

<sup>a</sup> TM band 6 was acquired at 120 m resolution, but products are resampled to 30-m pixels

<sup>b</sup> ETM+ band 6 is acquired at 60 m resolution, but products are resampled to 30-m pixels

**Fig. 2** Flow chart depicting the adopted methodology for computing the runoff using SCS-CN method



watersheds. Analysis of storm event rainfall and runoff records indicates that there is a threshold which must be exceeded before runoff occurs. It is expressed mathematically (Eqs. 1–8) as follows [64]:

$$\frac{F}{S} = \frac{Q}{P-I} \tag{1}$$

where,  $F$  is actual retention after runoff begins, mm;  $S$  is watershed storage, mm ( $S \geq F$ );  $Q$  is actual direct runoff, mm;  $P$  is total rainfall, mm ( $P \geq Q$ );  $I$  is initial abstraction, mm.

The amount of actual retention can be expressed as:

$$F = (P-I)-Q \tag{2}$$

The initial abstraction defined by the NRCS mainly consists of interception, depression storage, and infiltration occurring prior to runoff. Necessity of estimating both parameters  $I$  and  $S$  in the above equation, the relation between  $I$  and  $S$  was estimated by analyzing rainfall

runoff data for many small watersheds [64]. The empirical relationship is:

$$I = (0.2 \times S) \tag{3}$$

Substituting Eq. 3 into Eqs. 1 and 2

$$Q = \frac{(P-0.2 S)^2}{P + 0.8 S} \quad (P > 0.2 S) \tag{4}$$

It is a rainfall runoff equation used by the NRCS for estimating depth of direct runoff from storm rainfall. The equation has one variable  $P$  and one parameter  $S$ .  $S$  is related to CN by:

$$S = \frac{25400}{CN} - 254 \tag{5}$$

where, CN is a dimensionless parameter and its value ranges from 1 (minimum runoff) to 100 (maximum runoff).

**Table 2** Classification of soils into hydrological soil groups [10]

Characteristics	Hydrologic Soil Groups (HSGs)			
	A	B	C	D
Infiltration rate	High	Moderate	Slow	Very slow
Texture	Sand/gravel	Moderately coarse to moderately fine	Moderately fine to fine	Clay
Depth	Deep	Moderately deep to deep	Moderately deep	Shallow over an impervious layer or clay pan or high water table
Drainage	Well to excess	Moderately well drained to well drained	Moderately drained to slow	Very slow
Water transmission	High	Moderate	Slow	Very slow
Remarks	Low runoff	Moderate runoff	Moderate runoff	High runoff

**Table 3** Classification of antecedent moisture conditions

AMC	Total 5 days antecedent rainfall (mm)	
	Dormant season	Growing season
I	<12.7	<35.6
II	12.7–27.9	35.6–53.3
III	>27.9	>53.3

Its determination is based on the following factors: hydrologic soil groups, land use, land treatment, and hydrologic conditions. The NRCS runoff equation is widely used in estimating direct runoff because of its simplicity and flexibility.

SCS soil scientists, classified soils into four hydrologic soil groups namely A, B, C and D (Table 2) [64], depending on infiltration, soil classification and other criteria. Land use and treatment classes are used in the preparation of hydrological soil cover complex, which are used to estimate direct runoff.

Antecedent moisture conditions (AMC) (Table 3) is an indicator of watershed wetness and availability of soil moisture storage prior to a storm, and can have a significant effect on runoff volume. Recognizing its significance, SCS has developed a guide for adjusting CN according to AMC based on the total rainfall in the 5-day period preceding a storm. Three levels of AMC are used in the CN method: AMC-I for dry, AMC-II for normal and AMC-III for wet conditions. Table 3 gives seasonal rainfall limits for these three AMC. The CN values are

documented for the case of AMC-II [64]. Based on the McCuen and Richard [24], the composite CN can be computed by using the following equation [64]:

$$CN_{II} = \frac{\sum A_i \times CN_i}{\sum_i^n A_i} \tag{6}$$

where,  $CN_{II}$  is the composite CN,  $CN_i$  is the CN from 1 to any number,  $A_i$  is the area with curve number  $CN_i$  and  $A$  is the total area of watershed.

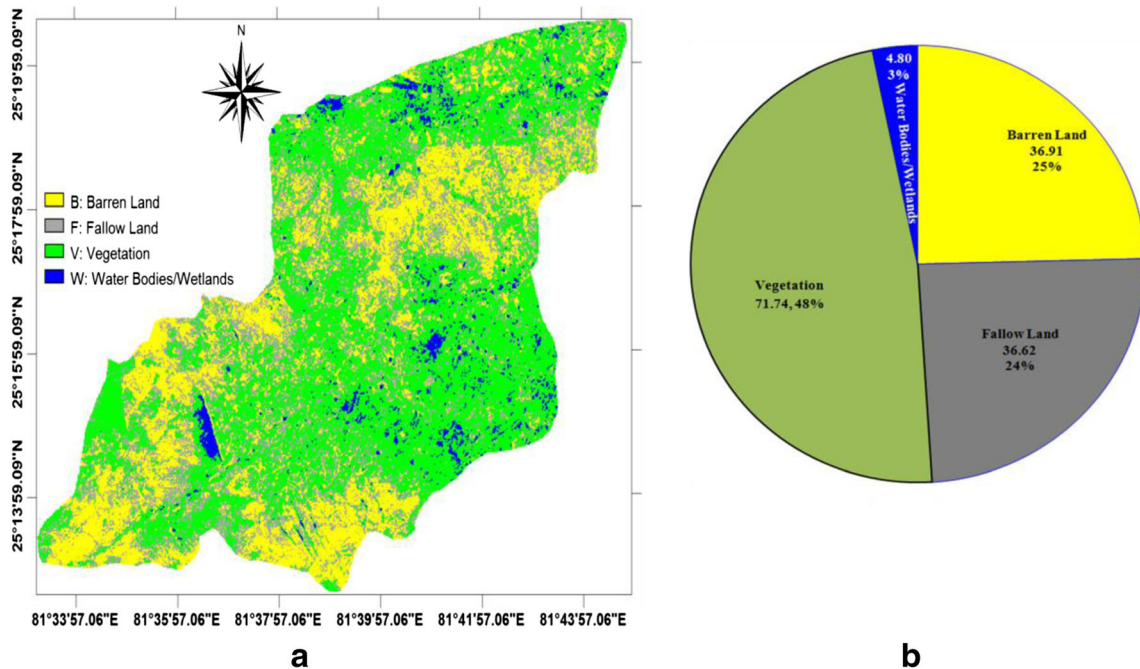
$$CN_I = \frac{42 \times CN_{II}}{10 - (0.058 \times CN_{II})} \tag{7}$$

$$CN_{III} = \frac{23 \times CN_{II}}{10 + (0.13 \times CN_{II})} \tag{8}$$

where,  $CN_{II}$  (normal condition),  $CN_I$  (dry condition) and  $CN_{III}$  (wet condition).

*Determination of CN Value*

The CN has potential of runoff estimation. Under the same precipitation condition, low CN values mean that the surface has a high potential to retain water whereas high value mean that the rainfall can be stored by the land surface only to a

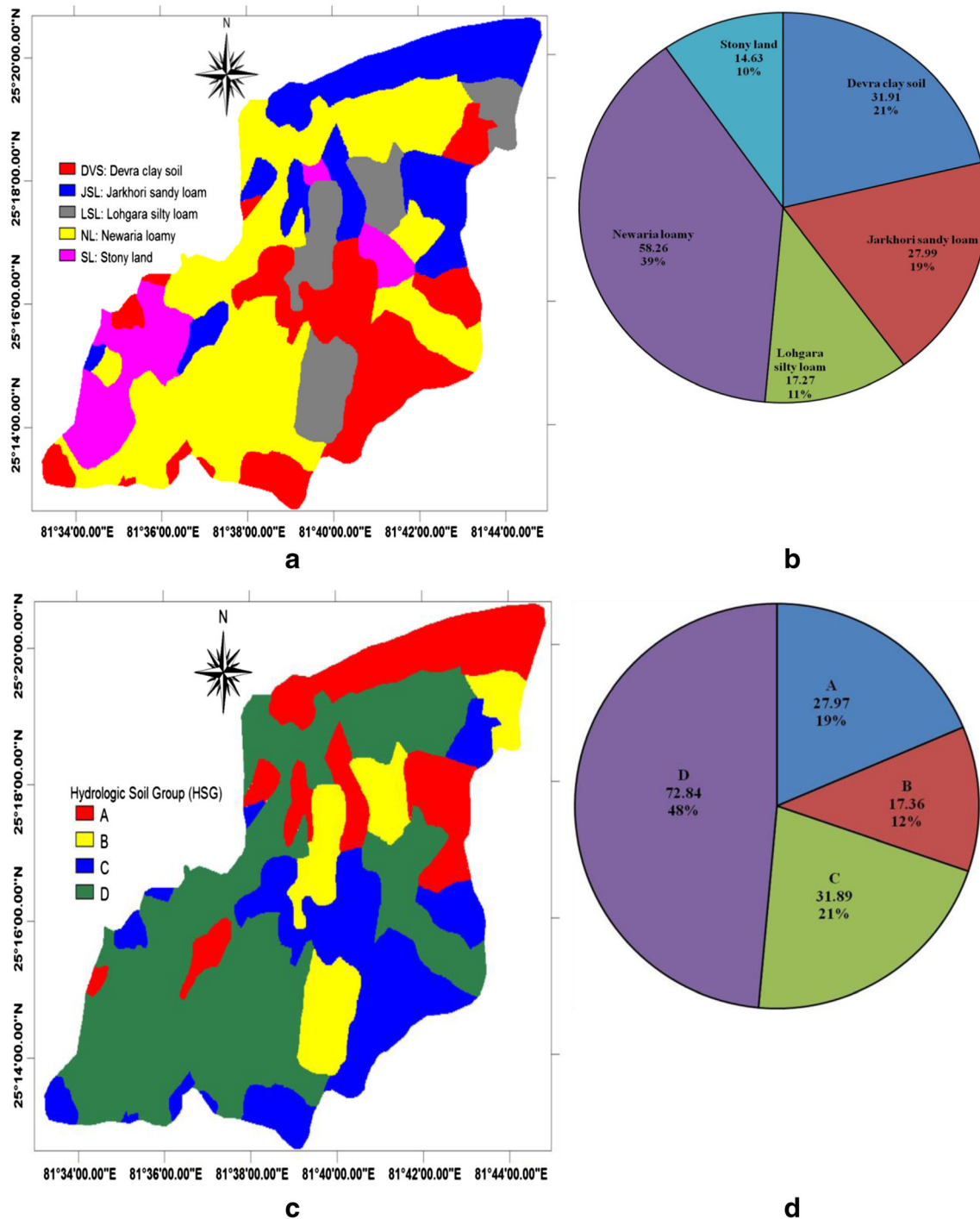


**Fig. 3** a Land use/land cover (LULC) map of the study area of year 2006, b the areal extent of different LULC types in Jhagrabaria watershed, Allahabad, (U.P.), India (area in km<sup>2</sup> and in %)

small extent. Therefore, areas with high CN value will produce a large amount of direct runoff and thereby contributed strongly to the flood peak.  $CN_{II}$  is the CN of normal condition,  $CN_I$  is the CN of dry condition,  $CN_{III}$  is the CN of wet condition. Calculated  $CN_n$  values from a given AMC condition are tested by plotting the rainfall and runoff data pairs, and the calculated rainfall runoff curves.

*SCS-CN Method for Runoff*

Several methods are available for estimation of runoff. Among them, the United States Department of Agriculture (USDA), SCS-CN method is the most popular and widely used [18, 39]. Simplicity, predictability, stability and its reliance on only one parameter namely the CN

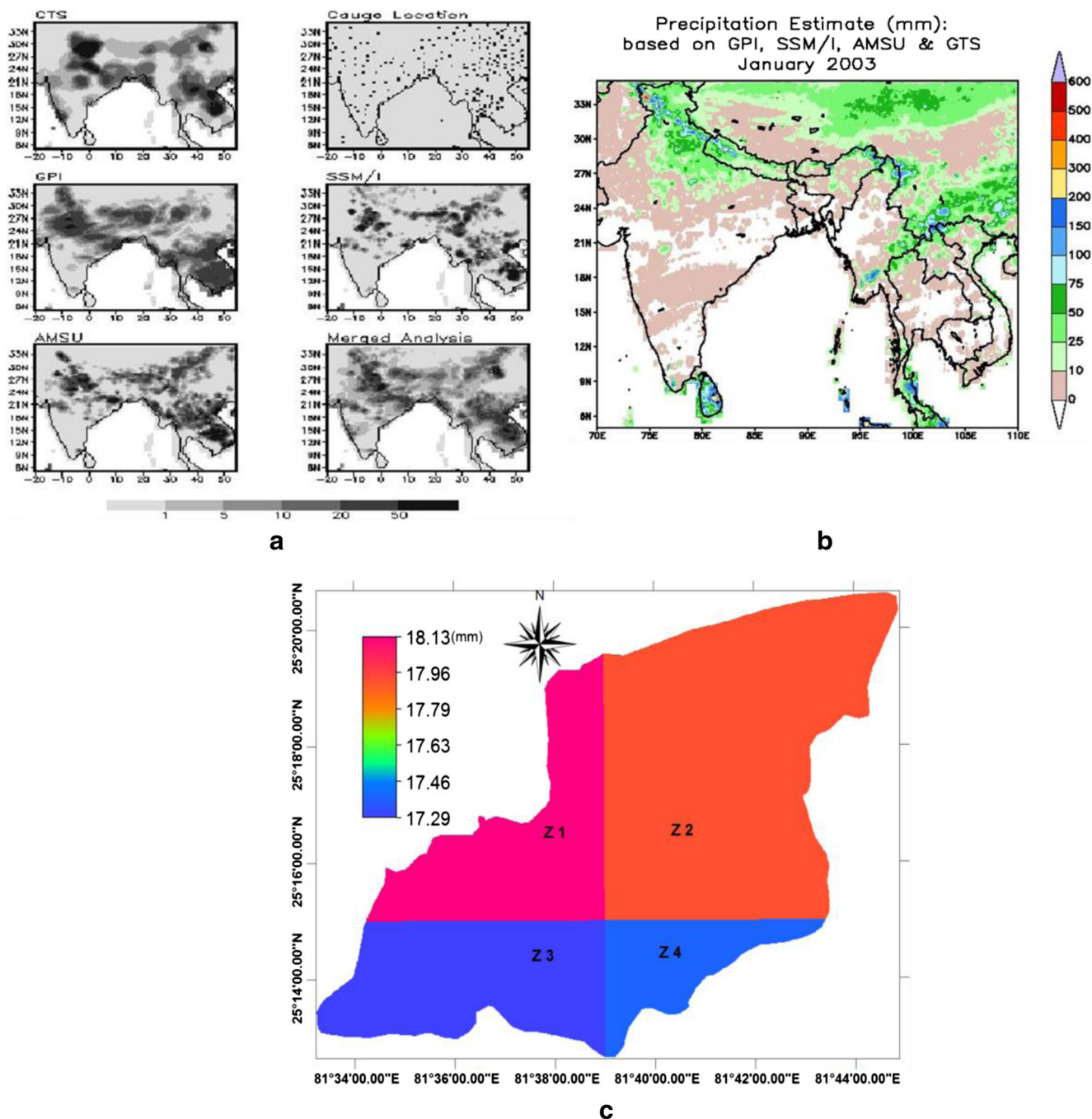


**Fig. 4** a Soil types in the study area and their spatial extent of distribution, b areal expenses of different soil types in the study area (km<sup>2</sup> and in %), c the hydrologic soil groups (HSGs) map and d areal expenses of different HSGs in the study area (km<sup>2</sup> and in %)

have few advantages of this method. LULC classes can be integrated with the HSGs of the sub basin in GIS environment and the weighted CN can be estimated. These estimated weighted CN for the entire area can be used to compute runoff. The main inputs required to the SCS-CN method are delineation of the watershed boundary, preparation of soil map, preparation of LULC map and AMC to estimate daily runoff.

*Model Verification*

In order to validate the SCS-CN method output, we used daily rainfall data of 14 years. The observed runoff data was not available; hence, we used the rainfall data as the proxy data. The validity and feasibility of the SCS-CN method based on the GIS was verified by comparing the estimated runoff with daily rainfall peaks. The results showed that the hydrographs of the



**Fig. 5** a Showing an example of the merged analysis of daily precipitation (rainfall) for the month of July 20, 2001. The merged analysis presents similar spatial distribution patterns with those of satellite estimates while its

magnitude is close to the gauge-based analysis over areas with gauge data, **b** shows a final product after merging the inputs and **(c)** Shows rainfall (from satellite final product) of August 20, 2003

**Table 4** Monthly rainfall and runoff in cm of period 2002–2015

Month	2002			2003			2004			2005			2006			2007			2008		
	R	Q	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q	
Jan.	0.7	0.0	0.0	0.6	0.0	0.0	2.5	0.0	0.0	3.7	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.3	0.0	
Feb.	4.4	0.1	0.0	6.2	0.0	0.0	0.0	0.0	0.0	1.4	0.0	0.0	0.0	0.0	0.0	6.2	0.2	0.0	0.5	0.0	
Mar.	0.2	0.0	0.0	0.3	0.0	0.0	0.0	0.0	0.0	3.1	0.0	0.0	3.0	0.0	0.0	2.6	0.0	0.0	0.1	0.0	
Apr.	0.1	0.0	0.0	1.9	0.0	0.0	2.2	0.0	0.0	0.8	0.0	0.0	4.8	0.1	0.0	0.6	0.0	0.0	2.7	0.0	
May.	2.6	0.0	0.0	0.1	0.0	0.0	1.3	0.0	0.0	1.7	0.0	0.0	1.4	0.0	0.0	1.9	0.0	0.0	0.4	0.0	
Jun.	9.1	0.5	0.1	11.7	1.3	0.1	20.4	4.0	0.2	14.4	5.6	0.4	10.3	0.2	0.0	11.6	0.3	0.0	33.2	7.3	
Jul.	12.1	0.5	0.0	21.8	2.3	0.1	18.3	0.9	0.1	39.4	9.9	0.3	42.4	12.5	0.3	25.6	3.5	0.1	44.9	15.8	
Aug.	32.5	9.9	0.3	32.3	6.8	0.2	22.7	1.8	0.1	32.9	13.6	0.4	16.0	1.6	0.1	35.3	6.9	0.2	39.7	12.5	
Sep.	21.7	5.9	0.3	48.2	12.1	0.3	18.4	2.3	0.1	16.0	2.8	0.2	14.2	2.5	0.2	20.7	9.6	0.5	6.9	0.8	
Oct.	5.2	0.0	0.0	0.4	0.0	0.0	0.3	0.0	0.0	1.4	0.0	0.0	5.1	0.1	0.0	0.9	0.0	0.0	1.2	0.0	
Nov.	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.4	0.0	0.0	0.0	0.0	0.0	0.2	0.0	
Dec.	2.1	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	1.0	0.0	0.0	0.2	0.0	0.0	0.9	0.0	0.0	0.0	0.0	
Total	90.8	17.0	0.19	123.7	22.6	0.18	86.2	9.0	0.10	115.9	31.9	0.27	97.7	17.0	0.17	106.5	20.6	0.19	130.2	36.4	

Month	2009			2010			2011			2012			2013			2014			2015			Average			Remarks	
	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q	C <sub>R</sub>	R	Q					
Jan.	0.0	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.4	0.0	0.0	0.2	0.0	0.0	4.5	0.0	0.0	4.6	0.0	0.0	20.7	0.0	0.0	Aug-03, Sep-03, Jul-05, Jul-06, Aug-07,	
Feb.	0.0	0.1	0.0	2.0	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	7.4	0.5	0.1	1.1	0.0	0.0	2.2	0.0	0.0	31.6	0.9	0.0	Jun-08, Jul-08, Aug-08, Jun-11, Jul-11,	
Mar.	0.0	0.9	0.0	0.0	0.0	0.0	0.3	0.0	0.0	0.3	0.0	0.0	1.8	0.0	0.0	0.2	0.0	0.0	8.9	0.7	0.1	21.9	0.7	0.0	Aug-11, Sep-11, Jul-12, Sep-12, Jun-13,	
Apr.	0.0	0.2	0.0	0.1	0.0	0.0	0.5	0.0	0.0	1.8	0.0	0.0	1.2	0.0	0.0	0.2	0.0	0.0	5.0	0.2	0.0	22.2	0.2	0.0	Jul-13, Aug-13, Jul-14 and Jul-2015.	
May.	0.0	5.0	0.1	0.9	0.0	0.0	4.5	0.0	0.0	0.1	0.0	0.0	0.1	0.0	0.0	0.6	0.0	0.0	0.3	0.0	0.0	20.9	0.1	0.0	All these months have above 30 cm rainfall (R)	
Jun.	0.2	7.2	1.0	2.8	0.0	0.0	32.6	14.9	0.5	6.9	0.2	0.0	38.5	12.1	0.3	6.2	0.1	0.0	7.0	0.1	0.0	211.8	47.5	0.2		
Jul.	0.4	23.6	4.0	28.6	4.8	0.2	35.6	10.3	0.3	37.2	8.4	0.2	32.9	6.6	0.2	37.1	11.2	0.3	33.3	7.4	0.2	432.8	98.1	0.2		
Aug.	0.3	11.0	0.6	24.2	2.4	0.1	31.4	6.8	0.2	31.1	6.1	0.2	54.1	17.1	0.3	24.1	6.5	0.3	27.1	5.2	0.2	414.5	97.8	0.2		
Sep.	0.1	25.0	8.5	20.9	2.0	0.1	32.7	8.7	0.3	32.2	12.8	0.4	11.3	0.2	0.0	15.1	1.7	0.1	4.3	0.0	0.0	287.6	70.0	0.2		
Oct.	0.0	7.2	0.4	8.2	1.6	0.2	0.0	0.0	0.0	0.6	0.0	0.0	30.5	15.2	0.5	8.4	0.9	0.1	2.4	0.0	0.0	71.8	18.3	0.3		
Nov.	0.0	2.7	0.0	2.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.0	0.0	0.0	5.7	0.0	0.0		
Dec.	0.0	2.1	0.0	0.0	0.0	0.0	0.3	0.0	0.0	2.2	0.0	0.0	0.0	0.0	0.0	0.3	0.0	0.0	0.0	0.0	0.0	9.3	0.0	0.0		
Total	0.28	86.1	14.5	0.17	89.8	10.9	0.12	138.1	40.6	0.29	114.8	27.4	0.24	178.0	51.8	0.29	98.0	20.4	0.21	95.0	13.5	0.14	1550.8	333.6	0.22	



estimated runoff process and the peak of rainfall coincided very well, even though the model may have higher uncertainties and less precision and low practicability due to non availability of the observed runoff.

## Results and Discussion

### Land Use/Land Cover (LULC)

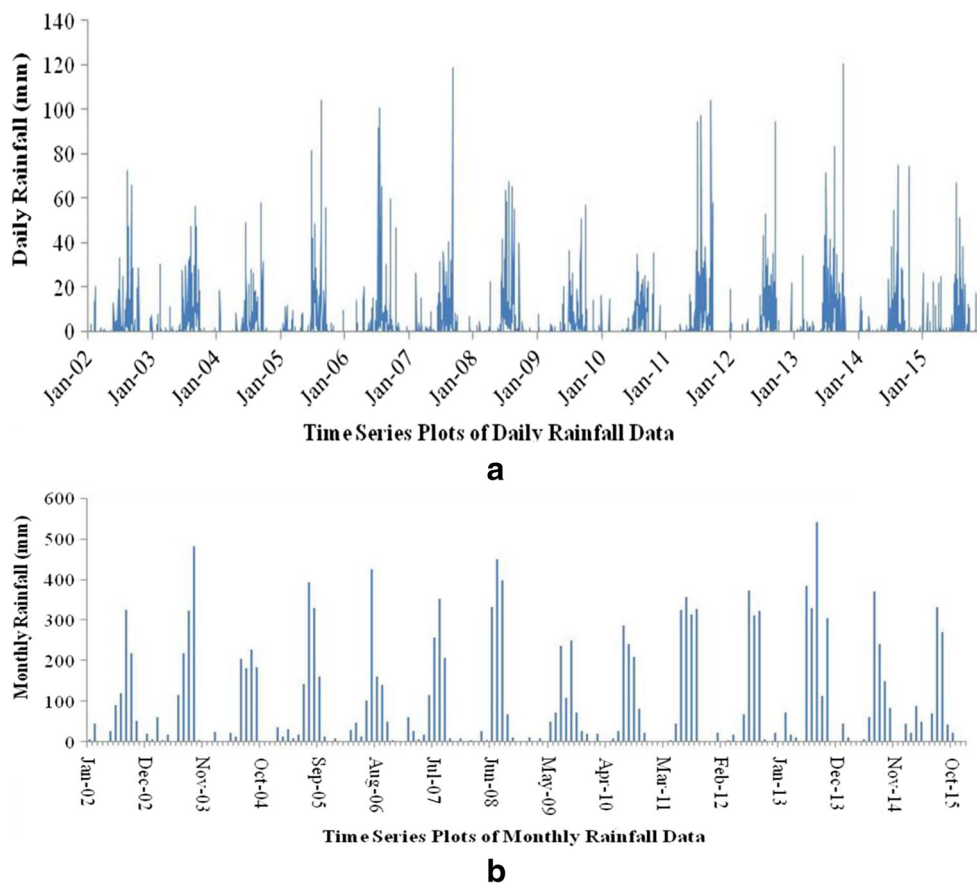
Overall accuracy was  $>86.79\%$  and Kappa statistics (0.80) [43]. LULC map of the watershed was classified into four classes namely; vegetation, fallow, barren and water bodies (Fig. 3a). It was observed that a 36.1% (or 24.59 km<sup>2</sup>) of total watershed area falls into barren land and 24.4% (or 36.62 km<sup>2</sup>) is allocated to fallow land. These two classes and the wetlands/water bodies mainly consists the non productivity land category (Fig. 3b). In our classification analysis, we found that not more than one-third of the study area is under vegetation. This is primarily due to recurrent water flooding and logging. The fallow, barren and fallow lands together forms the highest area, i.e. 48.99% of the total watershed area whereas nearly equal area (i.e. 47.81%) comes under vegetation condition. This is a sign of a model setting where the area exposed for erosion and have probability for water erosion [43]. Several studies have been conducted to

demonstrate the feasibility of interpreting the land use categories from EO data and further used as input data in a hydrologic modeling for estimating the runoff [1, 38, 58].

### Hydrologic Soil Groups (HSGs)

Based on infiltration rate, texture, depth, drainage condition and water transmission capacity, soils have been classified into different hydrologic soil groups: A, B, C and D. The criteria adopted for such classification is illustrated in Table 3 [10]. The soil taxonomic classes mainly consisted of Jarkhori sandy loam, Devra clay soil, Newaria loamy, stony land and Lohgara silty loam (Fig. 4a). The soil of area is divided into five different categories (according to NBSS and LUP). The watershed is mainly dominated by Newaria loam patties with 58.26 km<sup>2</sup> (or 38.82%) area, Devra clay soils with 31.92 km<sup>2</sup> (or 21.27%) and Jarkhori sandy loam soils 18.65% (or 27.99 km<sup>2</sup>) (Fig. 4b). Figure 4b, suggests that the soil of whole watershed is vulnerable to soil erosion during runoff of a high degree due to existence of sand fraction in large quantities. The stoniness of the land (14.63 km<sup>2</sup> or 9.75%) will act as barrier however leading to generation of higher amount of rainfall but prevent the soil loss from runoff. These classes help in development of hydrologic soil groups (HSG) (Fig. 4c) with help of standard guide line (Section 2C-

**Fig. 6** a Daily (January-December) rainfall time series at study area from 2002-2015, b Monthly (January-December) rainfall time series at study area from 2002-2015



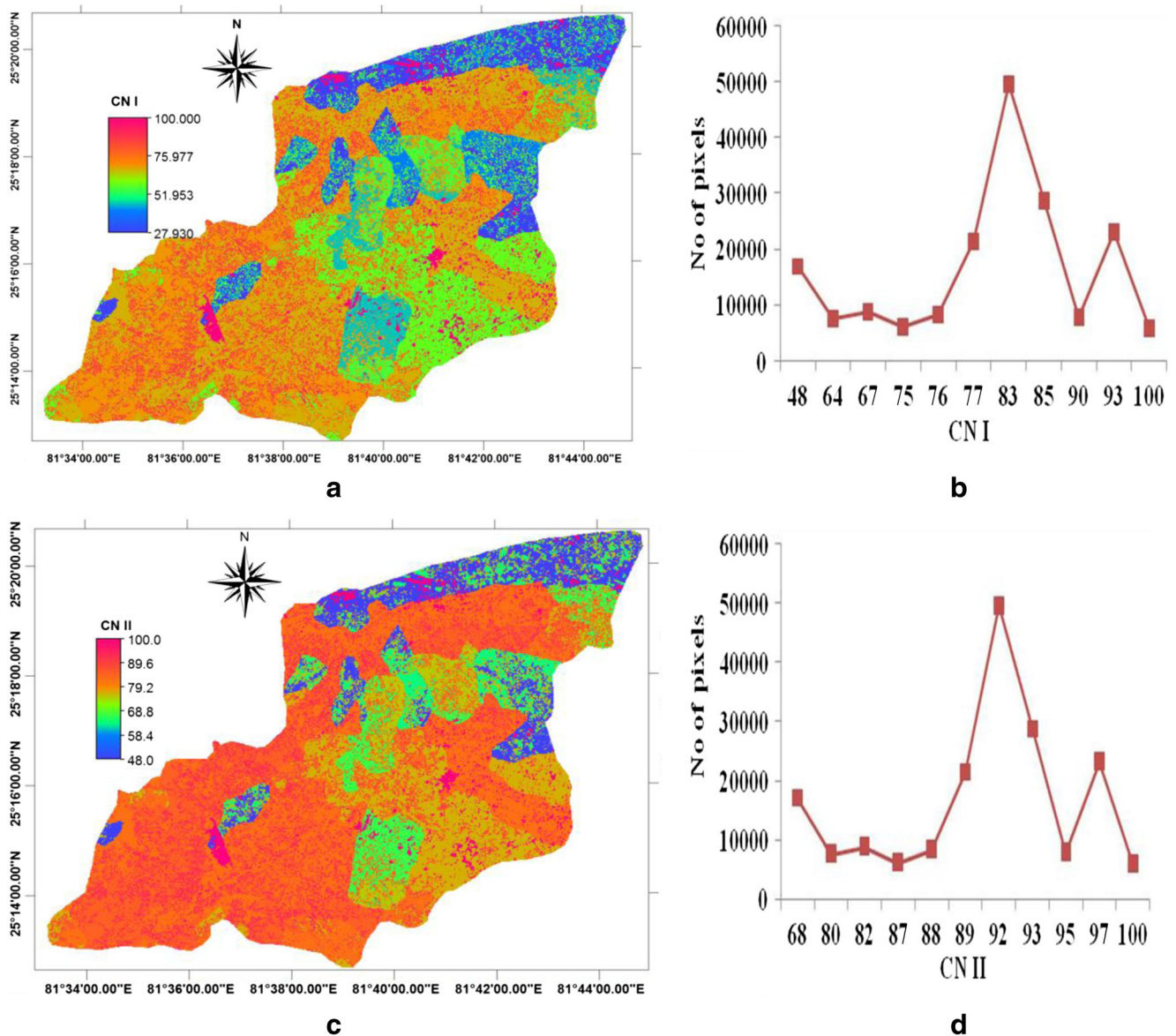
5—NRCs TR-55 Methodology) [15, 65]. The area under a different HSG in study area was estimated. Figure 4 d areal expenses of different HSGs in the study area (km<sup>2</sup> and in %). Overall, the HSG ‘D’ occupies the major portion of the study area. Thus, this indicates that the study area has very slow runoff potential but the HSG ‘A’ also occupies the large portion of area hence area have high runoff.

**Rainfall (2002–2015)**

The daily rainfall data of period 2002–2015 is shown in Fig. 5a and b. Figure 5c showed rainfall map of 20 August 2003, according to it the area comes under four zones (each zone represents to pixel value or rainfall value of 20 August 2003), and

therefore it is not required to draw THESSIEN polygons because rainfall is in already distributed format. The area is characterized by low, irregular and erratic rainfall pattern. Minimum annual runoff was recorded in the year 2004 with 86.2 cm (drought year over India) and 2002 with 90.8 cm (due to drought year) and annual runoff in the year 2013 with 51.63 cm (Table 4).

Figure 6a and 6b showed rainfall of per day and monthly rainfall of 14 years. In general, there is low rainfall (<60 cm annually) in the year 2004, 2009, 2010 and 2015. Hence, severe to moderate drought was occurred in 2004, 2009, 2010 and 2015. The monthly rainfall, runoff, and runoff coefficient of period 2002–2015 is presented in Table 4 and Fig. 6b. It was observed that highest monthly rainfall event were recorded in



**Fig. 7** a The Curve Number (CN<sub>I</sub>) map, b corresponding histogram c Curve Number (CN<sub>II</sub>) map d corresponding histogram e Curve Number (CN<sub>III</sub>) map f corresponding histogram and g estimation of surface runoff amounts from storm water

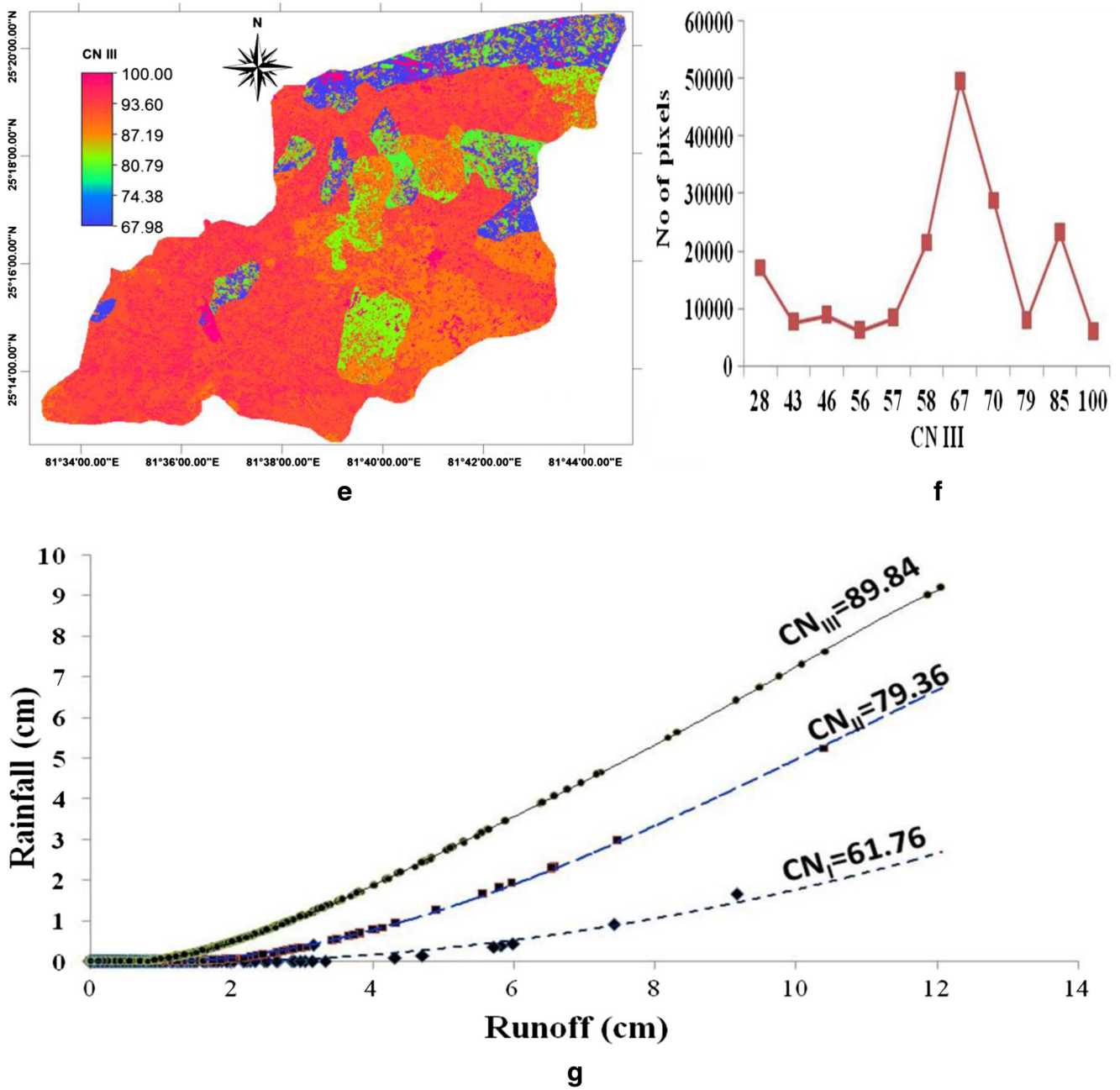


Fig. 7 (continued)

August 2013, and September 2003 with 54.14 cm (and 17.19 cm runoff) and 48.19 cm (12.10 cm runoff), respectively. Table 4 and Fig. 6b showed that September 2015 was recorded lowest monthly rainfall as 4.32 cm (during the monsoon). This trend coincides with moderate drought (less amount of rainfall or less annual rainfall 95.0 cm). Further, analysis showed that October rainfall was highest monthly rainfall as 30.5 cm (with 15.2 cm runoff) in 2013, 7.4 cm in February 2013, 8.9 cm in March 2015 and 5.0 cm in April 2015 in non monsoon season. The result shows slight level of variation especially in September monthly rainfall from 2003 to 2015.

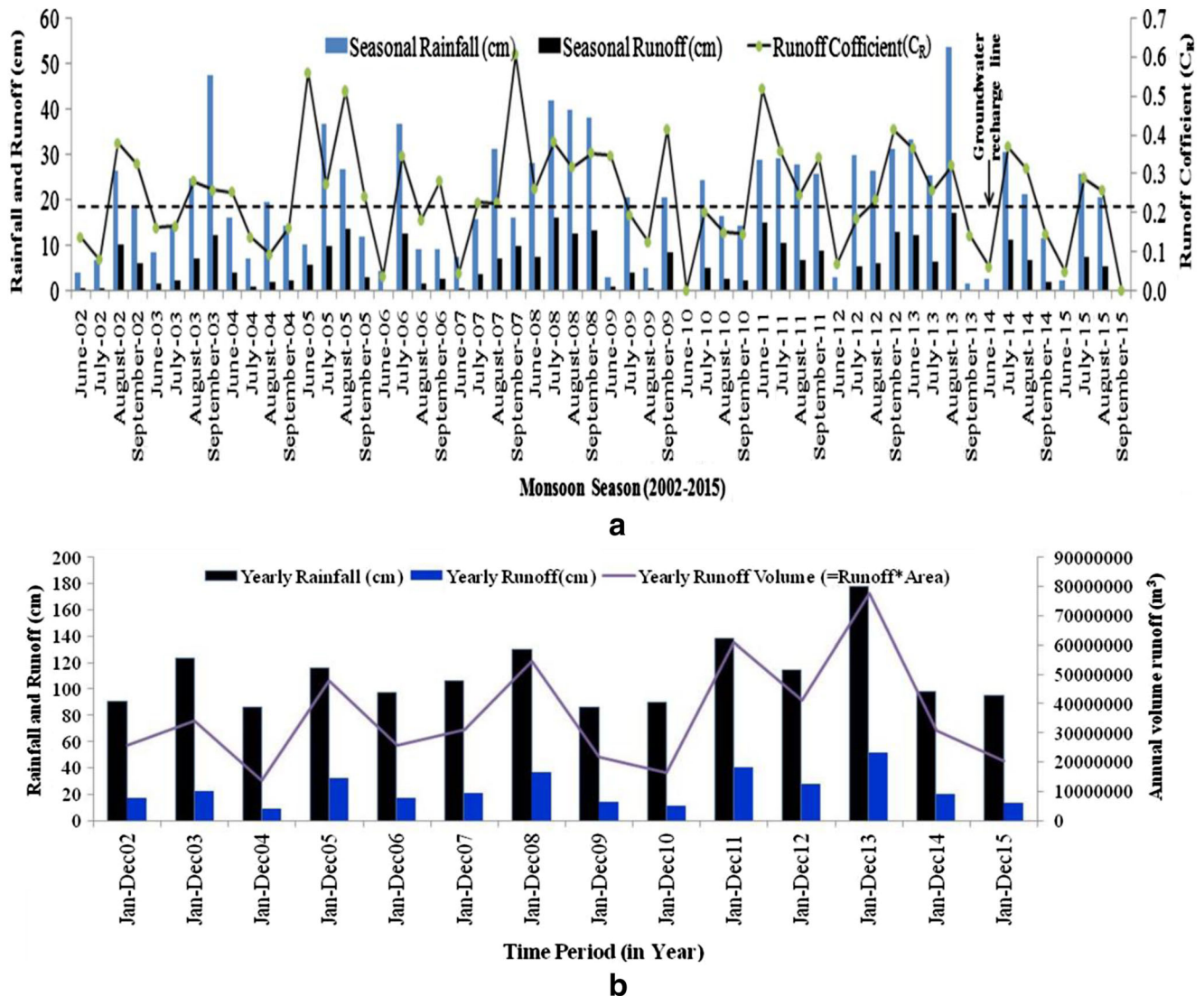
**Runoff and Runoff Coefficient (2002–2015)**

CN<sub>II</sub> is the CN of normal condition (Fig. 7a and b), CN<sub>I</sub> is the CN of dry condition (Fig. 7c and d), CN<sub>III</sub> is the CN of wet condition (Fig. 7e and f). Figure (7a–f) is showing destitution of CN<sub>n</sub> at spatial extent and corresponding histogram showing destitution of CN<sub>n</sub> at pixels wise ( $n = I, II$  and III) in images. Calculated CN<sub>n</sub> values from a given AMC condition are tested by plotting the rainfall and runoff data pairs, and the calculated rainfall runoff curves as shown in Fig. 7g.

Figure 8a represents seasonal trend, indicated the variability trends of runoff and runoff coefficient ( $C_R$ ). It was observed high  $C_R$  (is inversely proportional of rainfall) during June 2005 (0.56, because runoff was more than 55% of rainfall), August 2005, September 2007 (0.6, highest value of  $C_R$  due to higher value of runoff and higher value of runoff because of saturated soil due to high percentage of rainfall in August 2007) and June 2011 (0.52) because during these months runoff was high (>51% of total rainfall of the month) (Fig. 8a and Table 4). Thirty-three out of 56 months of monsoon season showed  $C_R$  above 0.2 (below 0.2, less runoff most and a large part of rainfall seep into soil) because during of these months, the area receives good amount of rainfall but most of the percentage of total rainfall has converted into runoff. Figure 8a has also revealed that during monsoon season from 2002 to 2015, 21 rainfall events have given more percolation time because during these rainfall events  $C_R \leq 0.2$ .

Above groundwater recharge line (>0.2) most of rainfall event gives more runoff or low absorption process therefore June and July rainfall improves groundwater recharge. From Table 4 and Fig. 8a has two times (June 2010 and September 2015)  $C_R$  reach to zero due to no rainfall.

From Fig. 8b and Table 5, the average annual rainfall, runoff, annual runoff coefficient and annual runoff volume of area were estimated as: 110.77 and 23.83 cm, respectively with average annual runoff coefficient as 0.204 of period 2002 to 2015 means 21.5% of average annual rainfall was used to recharge groundwater during the last 14 years and average annual volume runoff was  $3.57 \times 10^7 \text{ m}^3$  become wastewater from study area during 2002–2015. Table 5 has also revealed that total annual rainfall and runoff of study area was 1550.77 cm, and 333.63 cm respectively, but annual runoff coefficient (0.22) does not change much from average runoff coefficient (0.204). But total annual water loss in the form of



**Fig. 8** a Monsoon (2002–2015) rainfall, runoff and runoff coefficient of study area, b Annual (June–August) rainfall and runoff of time series at study area from 2002–2015

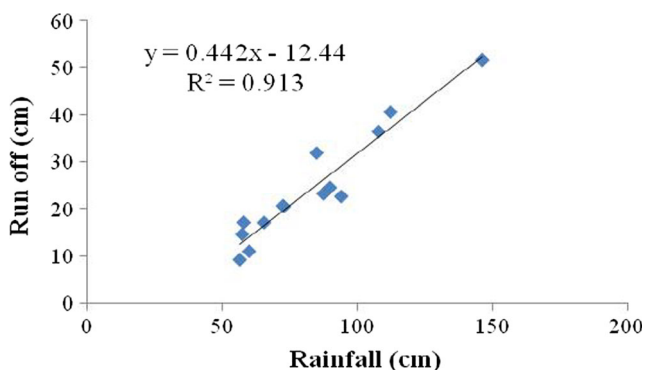
**Table 5** Annual rainfall and runoff depth and volume of watershed in cm of period 2002–2015

Year	Yearly rain(cm)	Yearly runoff (cm)	Runoff coefficient	Volume (m <sup>3</sup> ): runoff × area	Remarks
2002	90.829	17.01315	0.19	$2.55 \times 10^7$	2013 (178.04 <sub>Rainfall</sub> , 51.77 <sub>Runoff</sub> ), 2012 (114.76 <sub>Rainfall</sub> , 27.44 <sub>Runoff</sub> ), 2011 (138.14 <sub>Rainfall</sub> , 40.608 <sub>Runoff</sub> ), 2008 (130.19 <sub>Rainfall</sub> , 36.43 <sub>Runoff</sub> ), 2007 (106.51 <sub>Rainfall</sub> , 20.60 <sub>Runoff</sub> ), 2005 (115.88 <sub>Rainfall</sub> , 31.86 <sub>Runoff</sub> ), and 2003 (123.72 <sub>Rainfall</sub> , 22.58 <sub>Runoff</sub> ) All these years have/received more than 100 cm rainfall Area of watershed =150.06 km <sup>2</sup>
2003	123.721	22.57879	0.18	$3.38 \times 10^7$	
2004	86.164	9.042959	0.10	$1.35 \times 10^7$	
2005	115.876	31.86286	0.27	$4.78 \times 10^7$	
2006	97.703	16.98822	0.17	$2.54 \times 10^7$	
2007	106.509	20.60235	0.19	$3.09 \times 10^7$	
2008	130.187	36.42595	0.28	$5.46 \times 10^7$	
2009	86.114	14.50687	0.17	$2.17 \times 10^7$	
2010	89.799	10.86779	0.12	$1.63 \times 10^7$	
2011	138.13	40.608	0.29	$6.09 \times 10^7$	
2012	114.761	27.43582	0.24	$4.11 \times 10^7$	
2013	178.039	51.7656	0.29	$7.76 \times 10^7$	
2014	97.967	20.41944	0.21	$3.06 \times 10^7$	
2015	94.975	13.5143	0.14	$2.02 \times 10^7$	
Total	1550.77	333.63	0.22	$5.0 \times 10^8$	
Average	110.77	23.83	0.22	$3.57 \times 10^7$	

total annual volume runoff ( $5.0 \times 10^8 \text{ m}^3$ ) from study area was higher than average annual volume runoff ( $3.57 \times 10^7 \text{ m}^3$ ) during period 2002–2015.

**Rainfall Runoff Correlation Analysis**

After analysing the relationship between total rainfall, average surface runoff and average runoff volume, the relationship between rainfall and runoff was observed. The relationship between rainfall and the surface runoff is depends on number of factors relating to the watershed and climate, but in our analysis we assumed that other factors are uncountable due to lack of ground observed data sets. The correlation coefficient was found to be 0.91 (due to small study area) (Fig. 9), Tirkey et al. [62] also find good coefficient of determination (0.891) for large area therefore SCS-CN model can handle properly small as well as large study area.



**Fig. 9** Co-relationship between annual SCS-CN runoff (cm) and annual rainfall (cm)

**Conclusion**

The non availability of measured runoff arrest direct comparison between computed with the measured runoff at of Jhagrabaria ungauged agricultural watershed. However, based on HSGs obtained from soil classification, LULC, SCS-CN method and GIS made possible to measure runoff depth (cm or m) and runoff volume (m<sup>3</sup>) at ungauged watershed. The composite CN for normal condition is 79.35 (CN<sub>II</sub>), for the dry and wet conditions are 61.76 (CN<sub>I</sub>) and 89.84 (CN<sub>III</sub>), respectively. The average annual runoff depth of period 2002–2015 is equal to 23.83 cm (or 0.23 m), and the total area of the watershed is 150.06 km<sup>2</sup>, the average annual volume of water was  $3.58 \times 10^6 \text{ m}^3$ , which represents 21.50% of the average annual rainfall (110.77 cm or 1.11 m) of 14 years. The SCS-CN method satisfactorily computes the runoff for the available rainfall events in the Jhagrabaria watershed. The conventional hydrological data are generally not available for the purpose of the design and operation of water resources systems at the watershed or sub-watershed level, in such cases, EO and GIS can serve as better techniques for the estimation of runoff, watershed hydrology characteristics, geomorphology, slope, etc. Surface runoff and sediment losses are the two important hydrologic responses from the rainfall events occurring over the watershed systems. Rainfall generated runoff is very important in various activities of water resources development and management such as a: flood control and its management, irrigation scheduling, design of irrigation and drainage network, hydro power generation etc.

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