

Modeling of Compressive Strength Development of High-Early-Strength-Concrete at Different Curing Temperatures

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Abstract: High-early-strength-concrete (HESC) made of Type III cement reaches approximately 50–70 % of its design compressive strength in a day in ambient conditions. Experimental investigations were made in this study to observe the effects of temperature, curing time and concrete strength on the accelerated development of compressive strength in HESC. A total of 210 HESC cylinders of 100 × 200 mm were tested for different compressive strengths (30, 40 and 50 MPa) and different curing regimes (with maximum temperatures of 20, 30, 40, 50 and 60 °C) at different equivalent ages (9, 12, 18, 24, 36, 100 and 168 h). From a series of regression analyses, a generalized rate-constant model was presented for the prediction of the compressive strength of HESC at an early age for its future application in precast prestressed units with savings in steam supply. The average and standard deviation of the ratios of the predictions to the test results were 0.97 and 0.22, respectively.

Keywords: high-early-strength concrete, compressive strength, curing temperature.

List of symbols

A	Frequency factor
E	Activation energy in general
E_{eq}	Activation energy (=33,500 J/mol) used in the estimation of t_{eq}
f'_{cd}	Design compressive strength
k	Rate constant of concrete
k_r	Rate constant at reference temperature, T_r
R	Universal gas constant (=8.314 J/mol/K)
r	Reaction coefficient
S	Compressive strength of concrete at t_{eq}
S_u	Limiting strength of concrete
T_c	Temperature of concrete
T_{max}	Desired maximum temperature
T_r	Reference temperature
t	Real elapsed time
Δt	Time interval
t_{eq}	Equivalent age
t_{0r}	Age at the start of strength development at the reference temperature

1. Introduction

Steam curing accelerates hydration of concrete and allows the concrete to reach prestress levels of compressive strength in a short time. For precast prestressed concrete industry, method of steam curing is widely applied to concrete members made of Type I cement on behalf of the massive production and accelerated production rate in response to demand (Erdođdu and Kurbetci 1998; Erdem et al. 2003; Hanson 1963; Higginson 1961).

In architectural construction industry, a typical 3-6-3 curing regimes with a 24-h turnover period were typically exercised for the one unit of precast prestressed members made of Type I cement. It consists of 3-h preparation followed by 18 h of curing regime and 3-h cleaning periods. The 18-h curing regime is sequentially processed with a 3-h of delay followed by a 12-h of heat treatment, and a 3-h cooling period. The 12-h of steam curing comprises the temperature rise, constant maximum temperature, and descending temperature periods (ACI 517.2-2R-87 1992; Hanson 1963; Ramezaniyanpour et al. 2013). The prestressed strands are released at a partial completion of curing at about 15–18 h after concrete casting. At the release of prestressed strand, the concrete stresses induced by prestress transfer should be less than or equal to the allowable stresses (ACI 318-08 2008). That requires the compressive strength of concrete to reach approximately 70 % of the design compressive strength (f'_{cd} in MPa).

It has been known that the curing temperature and elapsed curing time have a significant effect on the strength development and its rate for concrete made of Type I (Oztekin 1984). In spite of the beneficial effects of steam curing on

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the acceleration of early strength development of concrete, steam curing can be regarded as costly and anti-eco-friendly as it consumes fuel and leaves carbon dioxide footprint during curing process by emitting.

The most beneficial effect of HESC made of Type III cement is its rapid development of compressive strength. About 50–70 and 100 % of its f'_{cd} of HESC can be reached in 1 and 7 days after casting at ambient temperature, respectively. If HESC is used for precast prestressed members, the accelerated development of compressive strength in HESC, compared with normal concrete with Type I cement, would require less fuel consumption and lessen the amount of carbon dioxide emissions by enabling a reduced steam curing regime.

For a rational and economical scheduling of steam curing regime, an appropriate measure is needed for the development of compressive strength of HESC at different curing conditions with different curing temperatures and ages. In this research, experimental investigations were performed to examine the effects of curing temperature, age and some other influential factors on the development of compressive strength in HESC with an emphasis on its strength development within 7 days (168 h). A series of regression analyses were performed on the test data to develop a generalized model for the prediction of compressive strength in HESC.

2. Experiments

2.1 Mix Proportions

Table 1 presents chemical compositions of Type III cement used in this study. A relatively higher chemical compositions of SO_3 , C_3A and C_3S in Type III cement with higher Blaine value than those in Type I cement contribute to a rapid development of concrete compressive concrete (Schindler and Folliard 2005). Mix proportions for the HESC used in this study were summarized in Table 2. Three different f'_{cd} of 30, 40, and 50 MPa were considered. Their target slump and air content were 200 ± 20 mm and 3 ± 1 %, respectively. Maximum size, specific gravity and fineness modulus of crushed gravel were 20 mm, 2.6 g/cm³ and 6.8, respectively. The specimens were first washed in order to minimize the effect of relative cleanliness on concrete properties in both the fresh and hardened states. River sand was used as the fine aggregate, which had specific gravity of 2.6 g/cm³ and a fineness modulus of 2.2. Superplasticizer (SP) was used to increase workability.

2.2 Preparation of the Specimens

Mixing and specimen preparation were conducted at room temperature and 50 ± 5 % relative humidity. Concrete is mixed by a 120-l capacity pan mixer. After 1 min of dry mixing with coarse aggregate, fine aggregate and cement, water and SP were added to the mixture in sequence and mixed for an additional 2 min. After mixing, the slump and air contents of the fresh concrete were measured according to the standard test specified in ASTM C143/C143M-10 (2010) and ASTM C231/C231M-14 (2014).

After obtaining the desired slump and air contents, the concrete was cast into 100 × 200 mm plastic cylindrical molds. In order to avoid agglutination between the mold and the concrete, the molds were cleaned in advance and brushed with a thin film of form oil. As illustrated in Fig. 1, water temperature contained in a plastic water bath was controlled to adjust the curing temperature of the cylinder according to the predetermined curing regimes. The water temperature was monitored with a thermocouple placed in the water and manually adjusted using a coil-type water heater submerged in the water bath. Using a thermocouple inserted into the mid-depth of the cylinder, change of concrete temperature during the curing period was measured.

Each batch cast fourteen cylinders. Before submerging the cylindrical molds containing fresh HESC into the curing water, each mold was placed into a waterproof plastic bag, and the top surface was covered with a plastic sheet to prevent evaporation of the water in the mold. The cylindrical molds were then placed into the water bath at the reference temperature of T_r ($=20$ °C in this study) for 3 h to allow the initial setting of HESC. After that, the water temperature was increased at a rate of 10 °C/h from 20 °C to the desired maximum temperature (T_{max}).

Preliminary tests showed that HESC developed a compressive strength close to its f'_{cd} value in 5 days and exceeds f'_{cd} value in 7 days of curing in an ambient environment. Based on that observation, the compressive strength of HESC was measured to the equivalent age of 7 days (168 h). Five different T_{max} of 20, 30, 40, 50 and 60 °C were considered in order to observe the effect of maximum temperature on the development of compressive strength in HESC at 7 different equivalent ages, 9, 12, 18, 24, 36, 100 and 168 h. The equivalent age (t_{eq} in h) in Eq. (1), suggested by Freiesleben and Pedersen (1977), was used in this study. Equation (1) is also recommended in ASTM C 1074-04 (2004). It takes into account the ratio of the maturity at concrete temperature (T_c in °C) to the one at T_r :

Table 1 Chemical and physical properties of Type III cement used in this study.

Chemical composition (%)						Mineral composition (%)						Density (g/cm ³)	Blaine (cm ² /g)
CaO	SiO ₂	Al ₂ O ₃	Fe ₂ O ₃	MgO	Na ₂ O	K ₂ O	SO ₃	C ₃ S	C ₂ S	C ₃ A	C ₄ AF		
62.2	19.5	5.9	3.1	2.8	0.2	0.8	4.1	49.2	18.8	10.3	9.5	3.12	4600

Table 2 Mix proportions.

f'_{cd} (MPa)	W/C	Slump (mm)	$\frac{S}{S+G}$	Weight per unit volume (kg/m ³)				SP/C
				Water (W)	Cement (C)	Sand (S)	Gravel (G)	
30	0.5	200	0.39	177	354	668	1053	0.01
40	0.41	200	0.37	177	433	606	1040	0.01
50	0.35	200	0.36	177	506	570	1022	0.01

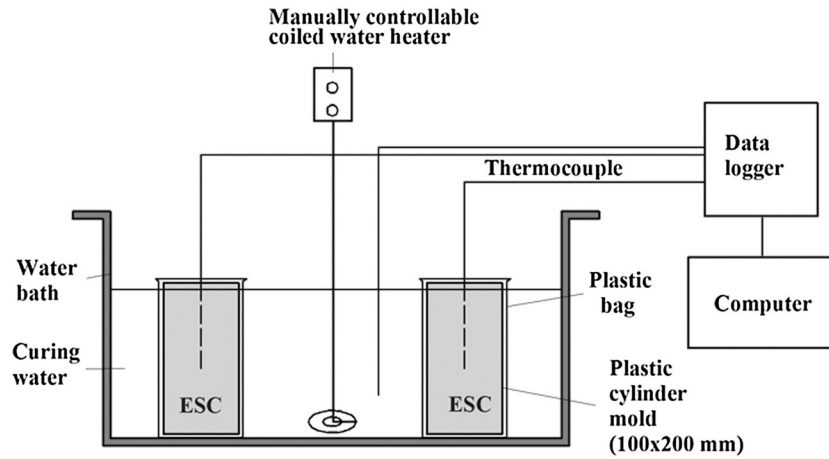


Fig. 1 Curing scheme of HESC in a water bath.

$$t_{eq} = \sum_0^t e^{-\frac{E_{eq}}{R} \left(\frac{1}{T_c+273} - \frac{1}{T_r+273} \right)} \cdot \Delta t \quad (1)$$

where, t is the real elapsed time (h), Δt is the time interval (h), E_{eq} is the activation energy ($=33,500$ J/mol) used in the estimation of t_{eq} , R is the universal gas constant ($=8.314$ J/mol/K) and T_c is the temperature of concrete ($^{\circ}\text{C}$).

Figure 2a illustrates the typical curing regime adopted in this study, which is composed of 3-h delay at ambient temperature, followed by different periods of temperature increase, maximum constant temperature, temperature decrease, and cooling at ambient temperature. All curing regimes began with a 3-h delay at 20°C for initial setting of the fresh concrete before the application of any additional heat. The rates of temperature increase and decrease in the concrete were maintained at 10 and -10°C/h , respectively, to prevent undesirable porosity or cracking (AASHTO 2004; Alexanderson 1972; CSA-A23.4-09 2014).

Table 3 lists 15 sets of test specimens. Test specimens are labeled using the form HCS n_1n_2 , where HC stands for HESC cylinder, S for f'_{cd} value ($S = L, M$ and H for $f'_{cd} = 30, 40$ and 50 MPa, respectively) and n_1n_2 indicates T_{max} . For each set for the identical conditions of f'_{cd} and T_{max} , a total of 14 cylinders – 7 sets of two replicas cast for the measurement of compressive strength at 7 different equivalent ages (9, 12, 18, 24, 36, 100 and 168 h)—were placed into the water. The time for a compression test was calculated at a certain t_{eq} by referring to the concrete temperature measured from the embedded thermocouple. Figure 2b presents the relationships between t_{eq} and real elapsed time (t) at strength measurement for each curing regime with a

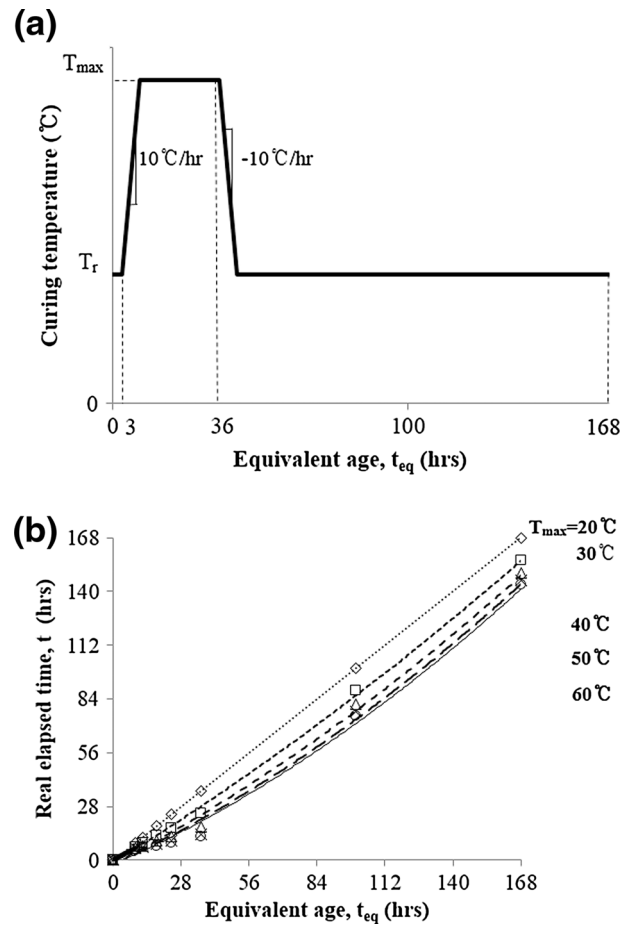


Fig. 2 Curing regimes with 5 different T_{max} : **a** a typical curing regime; **b** Equivalent age (t_{eq}) versus real elapsed time (t) at measurement for different T_{max} .

Table 3 Measured concrete strengths (S in MPa) at different values of T_{max} and t_{eq} .

Specimens		f'_{cd} (MPa)	T_{max} (°C)	Equivalent ages, t_{eq} (h)													
No.	Name			9		12		18		24		36		100		168	
1	HCL20	30	20	0.2	0.3	0.6	0.6	7.0	8.4	16.1	14.2	21.5	22.1	35.4	29.2	34.7	34.5
2	HCL30	30	30	0.1	0.1	1.3	1.0	7.0	6.8	15.6	15.6	21.5	22.9	32.6	36	33.1	33.4
3	HCL40	30	40	1.5	1.5	3.8	4.3	10.7	10.8	16.6	16.3	20.0	20.2	28.1	30.5	32.8	33.6
4	HCL50	30	50	1.0	0.0	0.5	0.7	8.4	6.3	12.6	13.6	20.3	18.8	32.3	32.6	33.9	33.2
5	HCL60	30	60	0.4	0.4	1.9	2.8	10.1	9.9	15.8	15.9	20.1	20.8	29.1	30.9	32.6	32.8
6	HCM20	40	20	0.4	0.4	3.9	4.3	23.1	23.5	27.6	29.0	34.5	35.8	41.7	43.0	45.5	43.9
7	HCM30	40	30	1.6	1.7	7.8	7.5	20.1	17.5	21.8	24.5	28.1	31.2	39.1	38.6	43.8	42.2
8	HCM40	40	40	2.0	2.3	8.9	9.7	19.2	20.7	23.7	25.0	29.0	30.0	38.0	39.8	41.8	42.5
9	HCM50	40	50	1.1	1.1	6.4	6.3	17.6	18.0	23.1	25.8	25.9	26.2	41.6	39.1	42.8	42.9
10	HCM60	40	60	1.2	1.1	6.5	8.2	18.1	17.8	23.9	26.3	29.0	32.5	42.8	42.6	43.8	43.7
11	HCH20	50	20	0.9	1.0	9.8	10.8	28.2	27.8	34.3	33.9	39.3	39.8	50.5	53.3	56.7	57.0
12	HCH30	50	30	0.3	1.0	4.4	3.6	25.2	24.5	31.0	32.3	41.7	40.7	49.1	46.3	54.3	53.9
13	HCH40	50	40	0.2	0.4	1.3	1.4	21.5	21.4	28.0	30.0	37.0	37.5	47.2	47.2	51.5	53.8
14	HCH50	50	50	0.2	0.5	2.7	2.9	17.4	16.3	29.8	30.9	34.1	34.3	46.7	45.0	51.5	50.0
15	HCH60	50	60	0.5	0.4	3.3	3.1	16.1	20.1	27.2	28.5	34.1	34.2	41.0	42.0	50.8	51.3

specific T_{max} . After the concrete reached the maximum temperature, the temperature was held constant until t_{eq} of 36 h, at which time the HESC was expected to reach 70 % of its f'_{cd} value. The temperature of the concrete was then reduced to 20 °C at a rate of -10 °C/h and maintained at 20 °C until performing the additional compression tests at the t_{eq} of 100 and 168 h.

2.3 Development of Concrete Compressive Strength

A set of two replica cylinders were removed from the water bath at the predetermined t_{eq} and stripped from their molds. The both top and bottom surfaces were ground shortly before they were tested in compression with a hydraulic servo-controlled compressive testing machine of 1000 kN capacity. The rate of loading was within the range of 0.25 ± 0.05 MPa/s (ASTM C39/C39M-14, 2014).

Table 3 tabulates the test results. Experimentally observed compressive strengths in Table 3, measured for different f'_{cd} and T_{max} , show that a majority of HESCs reached 70 % or more of their f'_{cd} values in 36 and 168 h in equivalent age, respectively. Figure 3 illustrates a trend of compressive strength development of HESC with different f'_{cd} for the same T_{max} . HESC with higher strength develops its compressive strength more rapidly at an early stage of curing than that with lower strength under the same T_{max} . At the later stage of curing, however, all concrete with different f'_{cd} values showed similar rates of strength increase. In general, concrete cured under higher T_{max} showed a more rapid increase in strength development in the early stage of curing,

but its rate decreased more rapidly at the later stage of the curing period.

Figure 4 shows the effects of T_{max} on the development of compressive strength in HESC with different $T_{max} = 20, 40$ and 60 °C for the same f'_{cd} value. For all cases, the rapid development of compressive strength was observed in the early stage of the curing period. The developing compressive strength, then, asymptotically approaches to the ultimate strength at the decreasing rate. Similar observations were also reported for concrete made of Type I cement.

When cured at a higher T_{max} , the compressive strength of HESC tended to develop at a higher rate in the early stage of curing. However, the rate of strength increase decreased more rapidly for the specimens cured under higher temperatures, resulting in an asymptotic approach of strength to a relatively lower limiting compressive strength with the increase in time compared with those cured under lower temperatures. This clearly shows that the curing temperature has a significant effect on the rate of hydration for HESC and leads to a crossover effect, commonly reported for the concrete with Type I cement. Crossover effect was caused by complex mechanisms involving stiffer and thicker build-up of hydrate shells, low permeability hydration products around the cement grains, retarded diffusion of the hydrate in the secondary reaction of cement hydration, non-uniform dispersion of reaction products within the pores of the hardening paste at higher temperature (Alexander and Taplin 1962; Carino and Lew 1983; Kjellsen et al. 1990; Kjellsen and Detwiler 1993; McIntosh 1956; Yi et al. 2005).

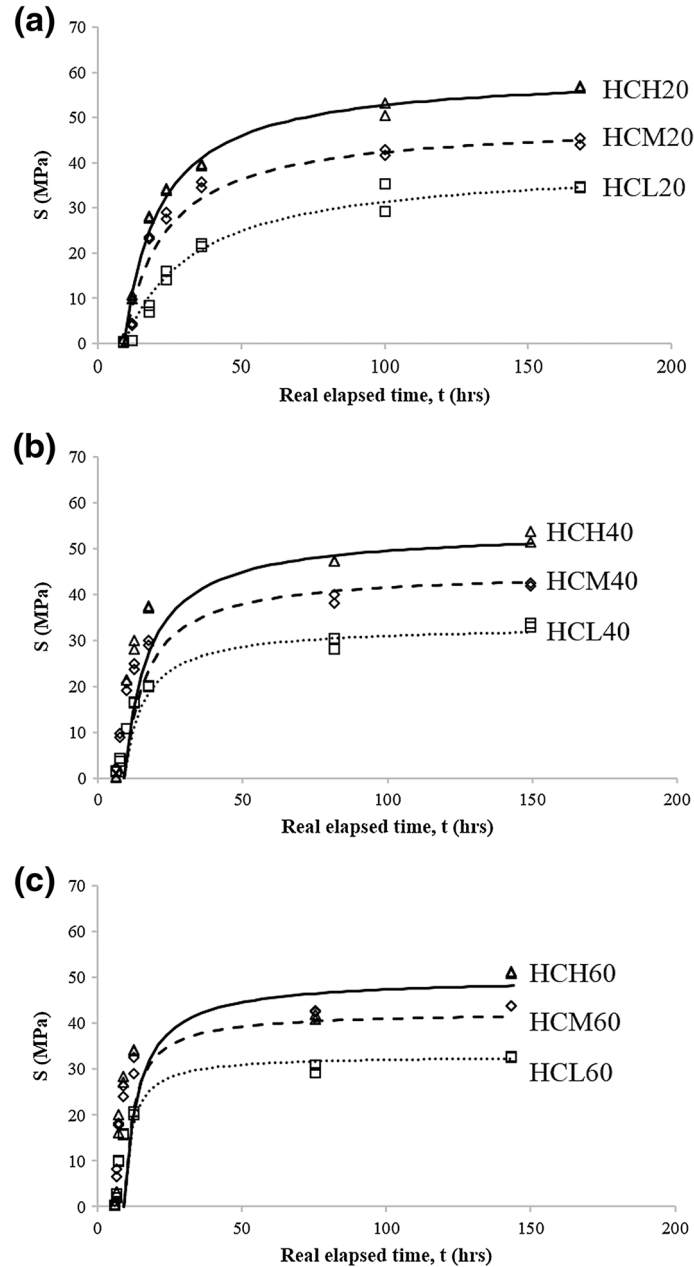


Fig. 3 Effect of f'_{cd} on concrete strength development (open square 30 MPa; diamond 40 MPa; and open triangle 50 MPa): **a** $f'_{cd} = 30$ MPa; **b** $f'_{cd} = 40$ MPa; **c** $f'_{cd} = 50$ MPa.

3. Prediction of ESC Compressive Strength

3.1 Previous Models: Review

A mathematical model for the rate of relative increase in concrete compressive strength with respect to its limiting compressive strength (S_u in MPa) was suggested by Bernhardt (1956) as given in Eq. (2). In Eq. (2), the reaction coefficient (r) reflects the retardation of continuous hydration as curing time elapses, and its value is taken as greater than zero. Equation (2) assumes that the rate of relative strength increase with respect to S_u decreases from its rate constant (k) to zero as the strength (S) approaches to S_u with the increase in elapsed curing time.

$$\frac{d\left(\frac{S}{S_u}\right)}{dt} = k \cdot \left(1 - \frac{S}{S_u}\right)^r \quad (2)$$

where, k is the rate constant of concrete (h^{-1}) and r is the reaction coefficient.

In Eq. (2), the rate constant, k , was adopted from the Arrhenius equation (Arrhenius 1889) for concrete. The k represents the number of collisions per unit time during the hydration reaction and is expressed as:

$$k = A \cdot e^{-E/[R \cdot (T_c + 273)]} \quad (3)$$

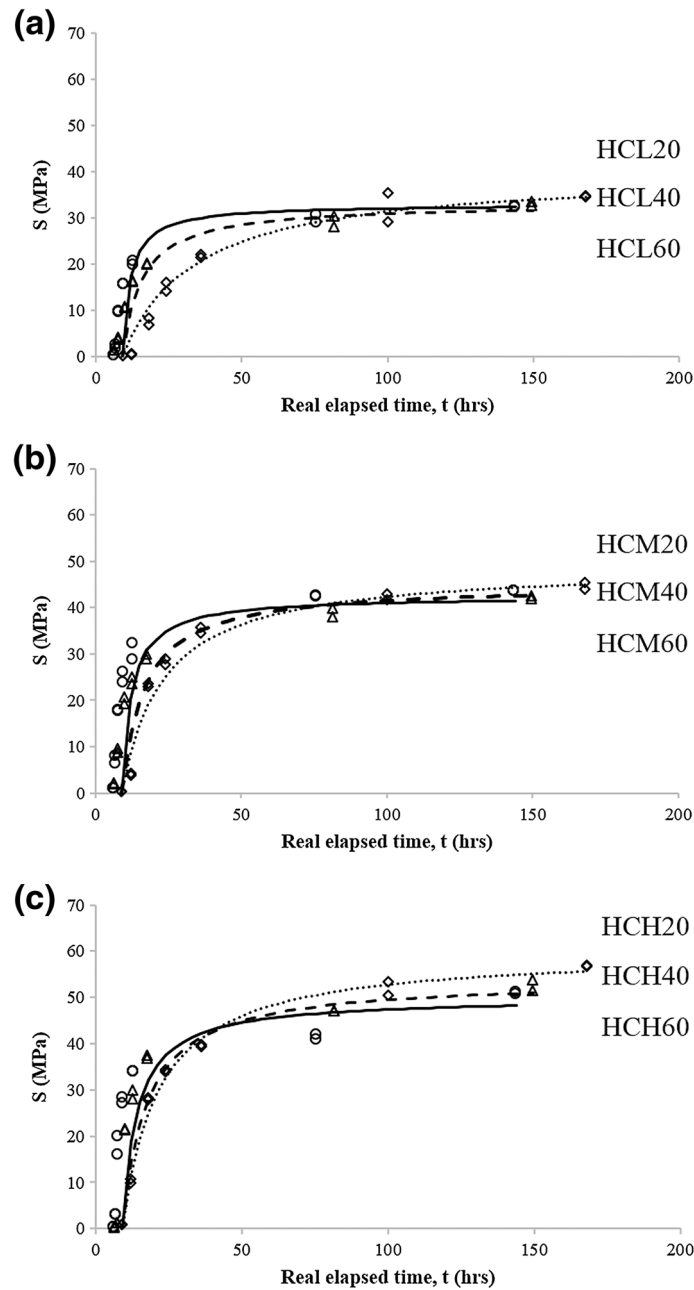


Fig. 4 Effect of T_{max} on concrete strength development (diamond 20 °C; open triangle 40 °C; and open circle 60 °C): **a** $T_{max} = 20$ °C; **b** $T_{max} = 40$ °C; **c** $T_{max} = 60$ °C.

where, A is the frequency factor (h^{-1}) and E is the activation energy in general (J/mol).

By integrating Eq. (2) with respect to time t and applying initial condition of $S(0) = 0$, two expressions for S corresponding to $r = 1$ or 2 can be obtained as given in Eq. (4):

$$S = \begin{cases} S_u \cdot (1 - e^{-kt}) & \text{if } r = 1 \\ S_u \cdot \left\{ 1 - \frac{1}{[1 + (r-1) \cdot k \cdot t]^{1/(r-1)}} \right\} & \text{if } r \neq 1 \end{cases} \quad (4)$$

Depending on the type of concrete, curing conditions or experimental parameters in consideration, different suggestions and modifications of Eq. (4) were suggested by different researchers (Carino and Tank 1992; Kim et al. 1998, 2001; Kwon et al. 2014; Yi et al. 2005). Bernhardt

(1956) and Carino (1984) suggested $r = 2$ after empirically investigating the development of strength in ordinary concrete exposed to different curing temperatures. Moon (1999) and Yi et al. (2005) suggested that a reaction coefficient r equal to 3 would estimate later-age strength development more accurately than that equal to 2.

Based on the basic equation with $r = 2$, the rate constant model given in Eq. (5) was proposed by Tank and Carino (1991) to estimate the relative strength gain of concrete based upon its t_{eq} .

$$S = S_u \cdot \frac{k_r \cdot (t_{eq} - t_{0r})}{1 + k_r \cdot (t_{eq} - t_{0r})} \quad (5)$$

where, k_r is the rate constant at reference temperature, T_r (h^{-1}) ($= A \cdot e^{-E/[R \cdot (T_r+273)]}$) and t_{0r} is the age at the start of strength development at the reference temperature (h).

In their work, quantitative information about the effect of temperature on the rate constant was obtained by curing specimens at various constant temperatures and through systematic analysis of the resulting strength gain data. Not only r , but also S_u as well as E and A in Eq. (3) have been functionalized with some influential factors after experimental observations. Tank and Carino (1991) showed that S_u is a function of curing temperature, supposing that the S_u decreases nearly linearly with curing temperature. Whereas Yi et al. (2005) observed that S_u depends mainly on curing temperature, Kwon et al. (2014) and Liao et al. (2008) suggested S_u as a function of temperature and humidity after their experimental observations. "Activation energy has been proposed as a function of temperature T_c by Freiesleben and Pedersen (1977) and Jonasson et al. (1995), T_c and t by Kim et al. (2001), the chemical components of the cementitious materials and Blaine value by Schindler (2004) and Poole (2006), the water-cement ratio (W/C), T_c , and degree of hydration by Abdel-Jawad (2006), and T_c and concrete age by Yang et al. (2016). Frequency factor was suggested as a function of both T_c and humidity by Liao et al. (2008) and a function of humidity only by Kwon et al. (2014)."

3.2 Development of the Model

In this study, the generalized model for the prediction of strength development of HESC was developed based on Tank and Carino's model in Eq. (5). Firstly, a series of regression analyses were performed for each set of 70 test data obtained for each f'_{cd} ($=30, 40$ or 50 MPa) cured under one of the 5 different curing regimes with T_{max} of 20, 30, 40, 50 and 60 °C. From the regression analyses, the best-fitting values of S_u , k_r and t_{0r} in Eq. (5) were identified for each data set. Case I in Table 4 tabulates the best-fitting values as well as the average (μ) and standard deviation (σ) of the ratios of predicted strengths (S_{hy}) to experimentally observed ones (S_{exp}) for $t_{eq} > t_{0r}$. It is interesting to observe that although the values of S_u and k_r show their dependency on f'_{cd} , the values of t_{0r} do not show their dependency on f'_{cd} . The maximum difference in t_{0r} was about 5.0 % at maximum with respect to their average value of 9.3 h (0.39 days). However, the statistical parameters, S_u and k_r , shown in Case I in Table 4, suggest the need for more refinement via influential factors.

Figure 5 shows comparisons between the model predictions and test results on the development of compressive strength of HESC with respect to t_{eq} for Case I. In general, the predicted values showed the general trend of strength development as a function. However, they tended to overestimate compressive strength development in the curing stage before t_{eq} less than 18 h with predicted strengths 1.64 times greater than the measured ones on average (Fig. 6).

Previous experimental studies showed that S_u , E and A were influenced by various factors, such as T_c , W/C , t , humidity, chemical components and Blaine. However,

relative humidity, chemical components and Blaine value were kept invariant during the test in this study. Consequently, these factors were excluded in this study when a general model was developed based on Eq. (5). A total of 15 separate regression analyses were performed for 5 different curing schemes applied to 3 different f'_{cd} values in order to determine the best-fitting coefficients for t_{0r} and S_u in Eq. (5) and A and E in Eq. (3), with $T_c = T_r (= 20^\circ\text{C})$ as given in Eq. (6). A constant value of $R = 8.314 \text{ J/mol/K}$ was used throughout. Case II in Table 4 tabulates the best-fitting values from 15 separate regression analyses.

$$k_r = A \cdot e^{-E/[R \cdot (T_r+273)]} \quad (6)$$

3.2.1 Frequency Factor, A

Regression analyses revealed that the maximum and minimum values of A were $1.005 \times 10^5 \text{ h}^{-1}$ for HCL50 and $1.010 \times 10^5 \text{ h}^{-1}$ for HCH20, respectively, as shown in Case II in Table 4 (Fig. 7a). The ratio of the maximum value of A to the minimum one was only 1.01, which would suggest that the effects of concrete strength and maximum temperature on A are marginal. From this observation, the value of A was determined to be constant at $1.0 \times 10^5 \text{ h}^{-1}$.

3.2.2 Age at the Start of Strength Development at T_r , t_{0r}

As observed in Case I in Table 4, values of t_{0r} from Case II in Table 4 remained between 8.2 and 10.2 h and did not show any clear dependency on either f'_{cd} or T_{max} (Fig. 7b). This observation corresponds to the finding of Kim et al. (2001) that t_{0r} does not depend on concrete strength. Using a constant value of $A = 1.0 \times 10^5 \text{ h}^{-1}$ and assuming a constant value of t_{0r} , independent of f'_{cd} and T_{max} , the best-fitting value of t_{0r} was found to be 9.02 h from regression analysis performed on 210 test data. From this observation, the constant values of $1.0 \times 10^5 \text{ h}^{-1}$ and 9.02 h (0.38 days) were assigned to A in Eq. (6) and t_{0r} in Eq. (5), respectively, in order to determine the factors that influence the remaining parameters in subsequent analyses.

3.2.3 Limiting Compressive Strength, S_u

Figure 8 shows the results of a regression analysis that obtained the ratios of the best-fitting value of S_u at T_{max} to that of S_u at T_r (S_{ur}) for 15 different cases with 3 different f'_{cd} and 5 different T_{max} values. Figure 8 clearly demonstrates the crossover effect, that is, a decrease in S_u with an increase in T_{max} , for all concrete strengths. Figure 8 also shows that, in general, S_u/S_{ur} is influenced by f'_{cd} at a given T_{max} . Test results showed that concrete strength obtained from different mix proportions resulted in different S_{ur} . Based on the test results, S_{ur} is expressed as a linear function of f'_{cd} in this study. To reflect the dependency of S_u/S_{ur} on T_{max} and f'_{cd} , the following regression equation is suggested to evaluate S_u in Eq. (5):

$$S_u = S_{ur} \times \left[-0.0032 \cdot T_{max} + (a_s \cdot f'_{cd} + b_s) \right] \quad (7)$$

Table 4 Statistical parameters with the best-fitting values of S_{u1} , k_r , t_{0r} , A and E and a function of β .

Cases	Specimens		Eq. no.	f'_{cd} (MPa)	T_{max} (°C)	No. of data	Regression parameters and β				Statistical parameters on S_{u1}/S_{u2}		
	No.	Name					S_{u1} (MPa)	k_r (h ⁻¹)	t_{0r} (h)	A ($\times 10^5$ hrs ⁻¹)	E (J/mol)	β	μ
I	1-5	CL20-60	5	30	All	70	39.8	0.04	9.6	-	-	1.41	1.23
	6-10	CM20-60		40	All	70	47.0	0.07	8.8	-	-	0.99	0.30
	11-15	CH20-60		50	All	70	56.8	0.07	9.4	-	-	1.34	1.09
II	1	CL20	5	30	20	14	44.0	0.031	9.7	1.006	36,541.3	1.66	1.54
	2	CL30			30	14	42.6	0.034	9.9	1.006	36,270.3	1.30	0.66
	3	CL40			40	14	41.1	0.036	8.2	1.006	36,154.5	1.01	0.15
	4	CL50			50	14	39.7	0.034	10.2	1.005	36,287.0	1.45	1.09
	5	CL60			60	14	38.3	0.042	9.3	1.006	35,789.3	1.11	0.32
	6	CM20		40	20	14	52.5	0.071	9.3	1.010	34,518.5	1.17	0.43
	7	CM30			30	14	50.8	0.054	8.4	1.009	35,185.0	1.02	0.07
	8	CM40			40	14	49.1	0.063	8.3	1.007	34,780.9	1.03	0.06
	9	CM50			50	14	47.4	0.060	8.8	1.008	34,900.1	0.96	0.23
	10	CM60			60	14	45.7	0.080	9.0	1.009	34,227.4	1.03	0.11
	11	CH20		50	20	14	62.4	0.076	8.9	1.010	34,340.3	0.97	0.19
	12	CH30			30	14	60.4	0.068	9.3	1.010	34,616.0	1.21	0.54
	13	CH40			40	14	58.3	0.059	9.6	1.009	34,952.6	1.69	1.70
	14	CH50			50	14	56.3	0.057	9.8	1.009	35,047.0	1.19	0.50
	15	CH60			60	14	54.2	0.063	9.6	1.008	34,791.3	1.19	0.50
III	1-15	All	5	All	All	210	Equation (7)	Equation (6)	9.02	1.000	Equation (8)	1.30	1.16
IV	1-15	All	10	All	All	210	Equation (7)	Equation (6)	9.02	1.000	Equation (8)	0.97	0.22

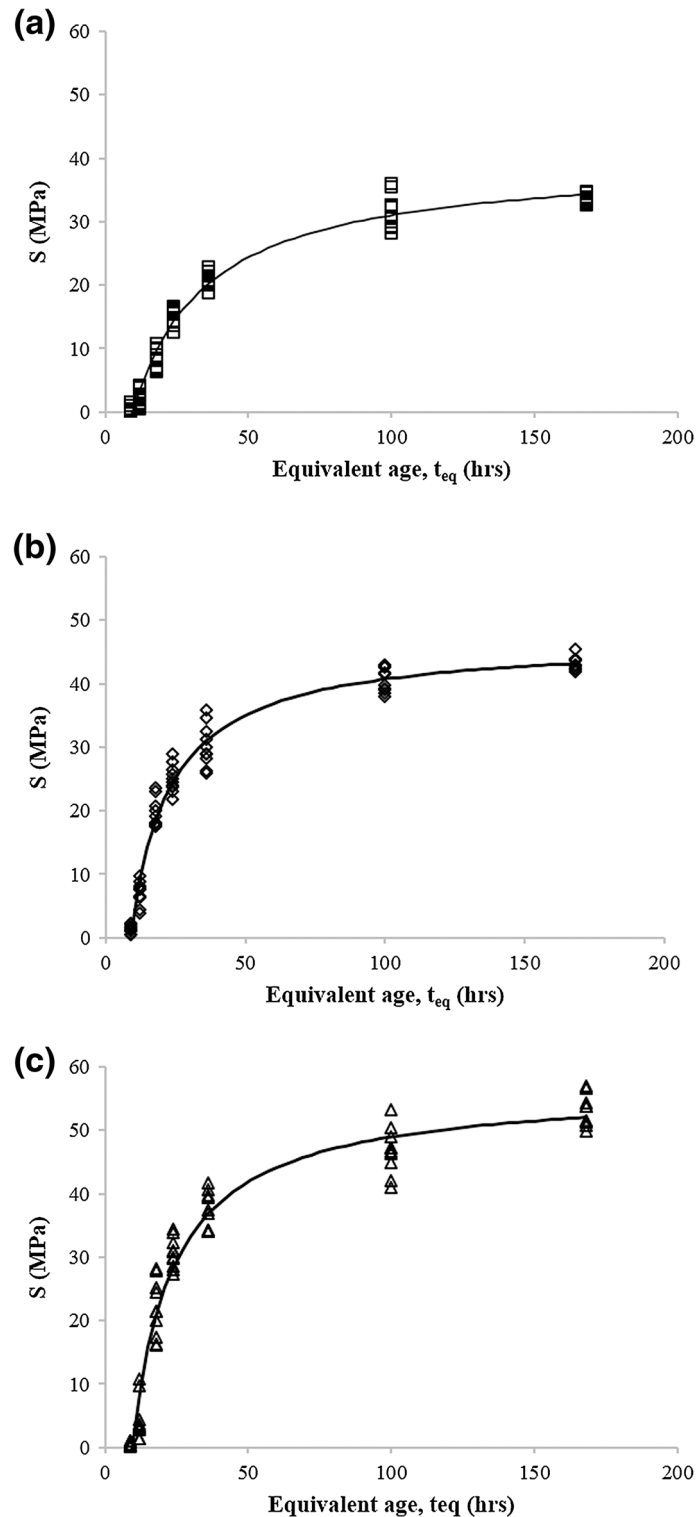


Fig. 5 Predictions of compressive strength development at different t_{eq} by Eq. (5): Case I in Table 4 (*open square* 30 MPa; *diamond* 40 MPa; and *open triangle* 50 MPa).

where, $S_{ur} = 0.95f'_{cd} + 16.0$, $a_s = -0.0005$ and $b_s = 1.0684$.

Equation (7) is in accordance with previous models: S_u as a function of T_{max} by Tank and Carino (1991); f'_{cd} and T_{max} by Yi et al. (2005); and T_{max} by Kwon et al. (2014) and Liao et al. (2008) if the relative humidity remains constant. Their models showed similar effects of T_{max} and f'_{cd} on S as experimentally observed in this study.

3.2.4 Activation Energy, E

Figure 9a exhibits that except for the cases when T_{max} changed from 20 to 40 °C for $f'_{cd} = 50$ MPa, E decreases in general with increasing T_{max} . Kim et al. (2001) and Jonasson et al. (1995) also observed that E decreased with increasing T_{max} . Figure 9a also shows that in general E decreases with an increase in f'_{cd} at a specific T_{max} . Those experimental results are also in accordance with previous observations

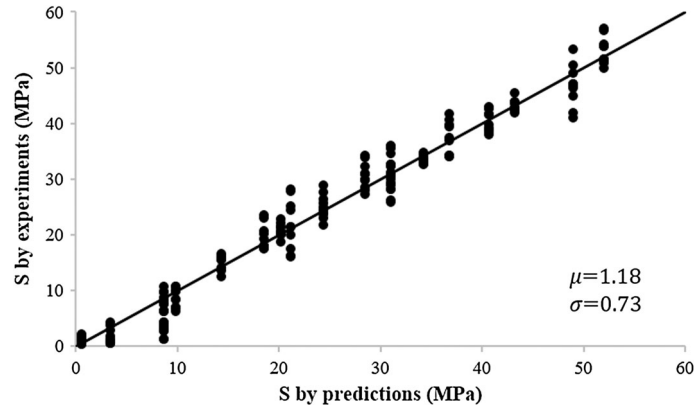


Fig. 6 Comparisons with 210 experimental results: Case I in Table 4.

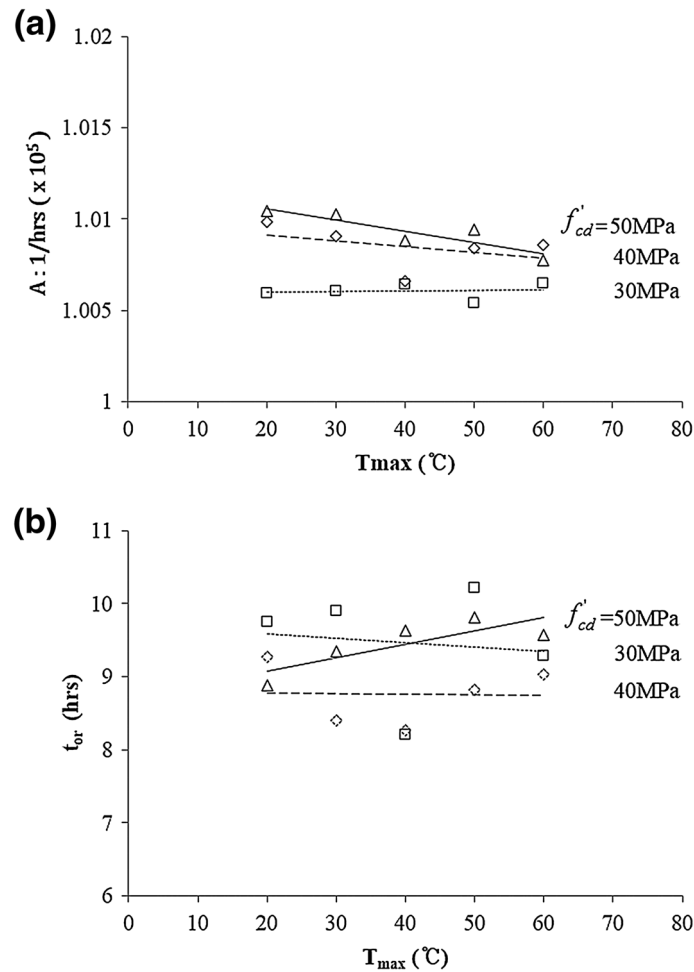


Fig. 7 Effects of T_{max} on A and t_{0r} : Case II in Table 4 (open square 30 MPa; diamond 40 MPa; and open triangle 50 MPa): a T_{max} versus A ; b T_{max} versus t_{0r} .

made by Kim et al. (2001), Kwon et al. (2014) and Abdel-Jawad (2006). Figure 9b illustrates that higher E was observed in general with lower T_{max} at a given value of W/C . This observation also agrees with the finding reported by Abdel-Jawad (2006). Based on the experimental observations, the following expression is suggested for E of HESC as a function of both T_{max} and W/C :

$$E(T_{max}, f'_{cd}) = a_e \times T_{max} + b_e \times W/C + c_e \leq E_{max} \quad (8)$$

where, $a_e = -47.2 \times W/C + 9.3$, $b_e = 10,541$ and $c_e = 31,308$.

Equation (5) was modified by substituting E in Eq. (8) into Eq. (6) and S_u in Eq. (7) into S_u in Eq. (5). Figure 10a shows a comparison between the predictions made by the modified model and the 210 experimental results (Case III in Table 3). The μ and σ of the ratios of the model predictions to experimental results were 1.30 and 1.16, respectively.

The predictions made by the modified model were shown to have a relatively large value of μ with a significant scatter by a large value of σ ($=1.16$). This was mainly due to overestimations on the compressive strengths of HESC at early equivalent ages less than or equal to 18 h as indicated in Fig. 10b for f'_{cd} equal to 30 MPa. The same trends of overestimation were also observed from the models developed for Case I. For f'_{cd} equal to 40 and 50, similar

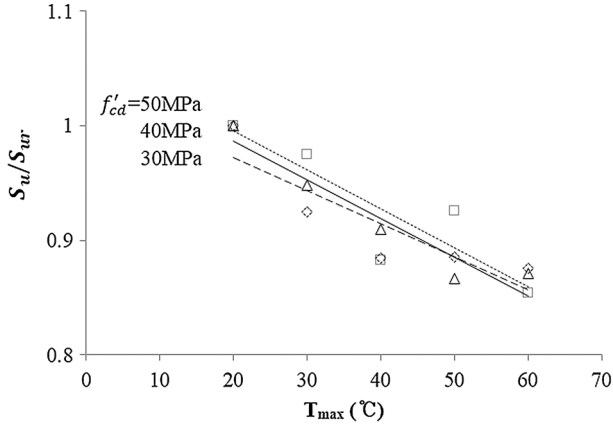


Fig. 8 Effects of T_{max} and f'_{cd} on S_u/S_{ur} with $A = 1.0 \times 10^5 \text{ h}^{-1}$ and $t_{0r} = 9.02 \text{ h}$ (open square: 30 MPa; diamond 40 MPa; and open triangle 50 MPa).

overestimations were also observed, especially for the t_{eq} less than or equal to 18 h. In order to improve model predictions at early age, a modification function for early age was introduced in Eq. (9) through regression analyses of the 210 test data. The function (β) is composed of a product of linearly decreasing and exponentially decaying functions with respect to the increase in t_{eq} .

$$\beta = \frac{S_{exp} - S_{thy}}{S_{exp}} = [1 - a_r \cdot 24 \cdot (t_{eq} - t_{0r})] \times e^{-24b_r \cdot (t_{eq} - t_{0r})} \quad (9)$$

where, $a_r = a_{b1}T_{max} + b_{b1}$ and $b_r = a_{b2}T_{max} + b_{b2}$.

In Eq. (9), a_{b1} and b_{b1} have different fitting values of 0.37, and 0.006 for $f'_{cd} = 30 \text{ MPa}$, 0.0002 and 0.0001 for $f'_{cd} = 40 \text{ MPa}$, and 0.28 and 0 for $f'_{cd} = 50 \text{ MPa}$, respectively. The coefficients of a_{b2} and b_{b2} also have different values of 0.016 and 0.21 for $f'_{cd} = 30 \text{ MPa}$, 0.15 and 0.006 for $f'_{cd} = 40 \text{ MPa}$, and 0.005 and 0.62 for $f'_{cd} = 50 \text{ MPa}$, respectively. Including a corrective function β in Eq. (9), the generalized predictive equation is presented in Eq. (10) for the development of compressive strength of HESC. In Eq. (10), A , R and t_{0r} take constant values of $1.0 \times 10^5 \text{ hrs}^{-1}$, 8.314 J/mol/K and 9.02 h , respectively. The expressions for S_u from Eq. (7) and E from Eq. (8) were used in Eq. (10).

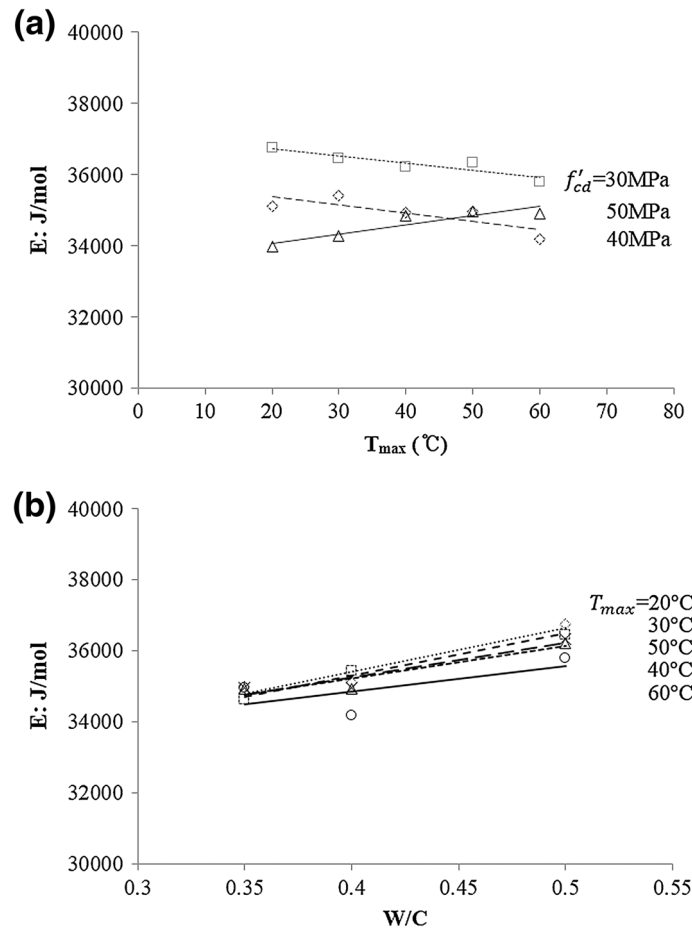


Fig. 9 Effects of T_{max} and W/C on E with $A = 1.0 \times 10^5 \text{ h}^{-1}$ and $t_{0r} = 9.02 \text{ h}$: **a** T_{max} versus E (open square 30 MPa; diamond 40 MPa; and open triangle 50 MPa); **b** W/C versus E (diamond 20 °C; open square 30 °C; open triangle 40 °C; times 50 °C; and open circle 60 °C).

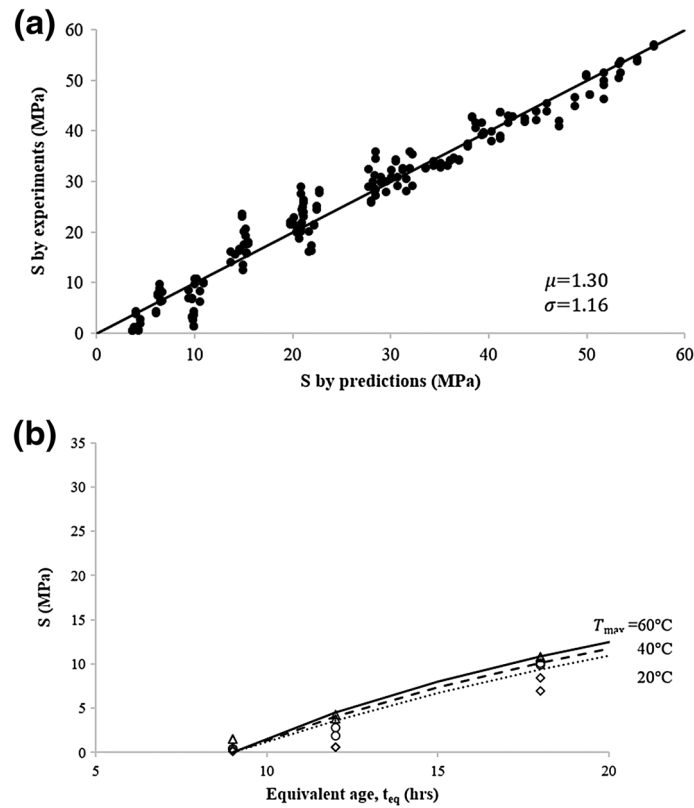


Fig. 10 Comparisons of predicted compressive strengths and experimental measured ones: Case III in Table 3: **a** Comparisons with 210 experimental results; **b** Overestimation by the modified model in early age ($f'_{cd} = 30$ MPa) (diamond: 20°C ; open triangle 40°C ; and open circle 60°C).

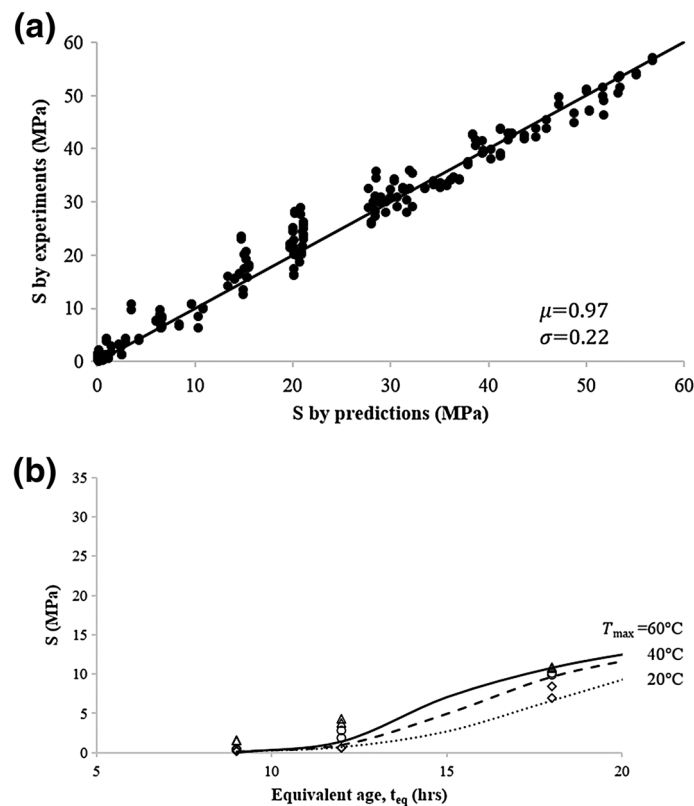


Fig. 11 Comparisons of predicted compressive strengths and experimentally measured ones: Case IV in Table 4: **a** Comparison with 210 experimental results; **b** Predictions of compressive strength by Eq. (10) in early age ($f'_{cd} = 30$ MPa) (diamond 20°C ; open triangle 40°C ; and open circle 60°C).

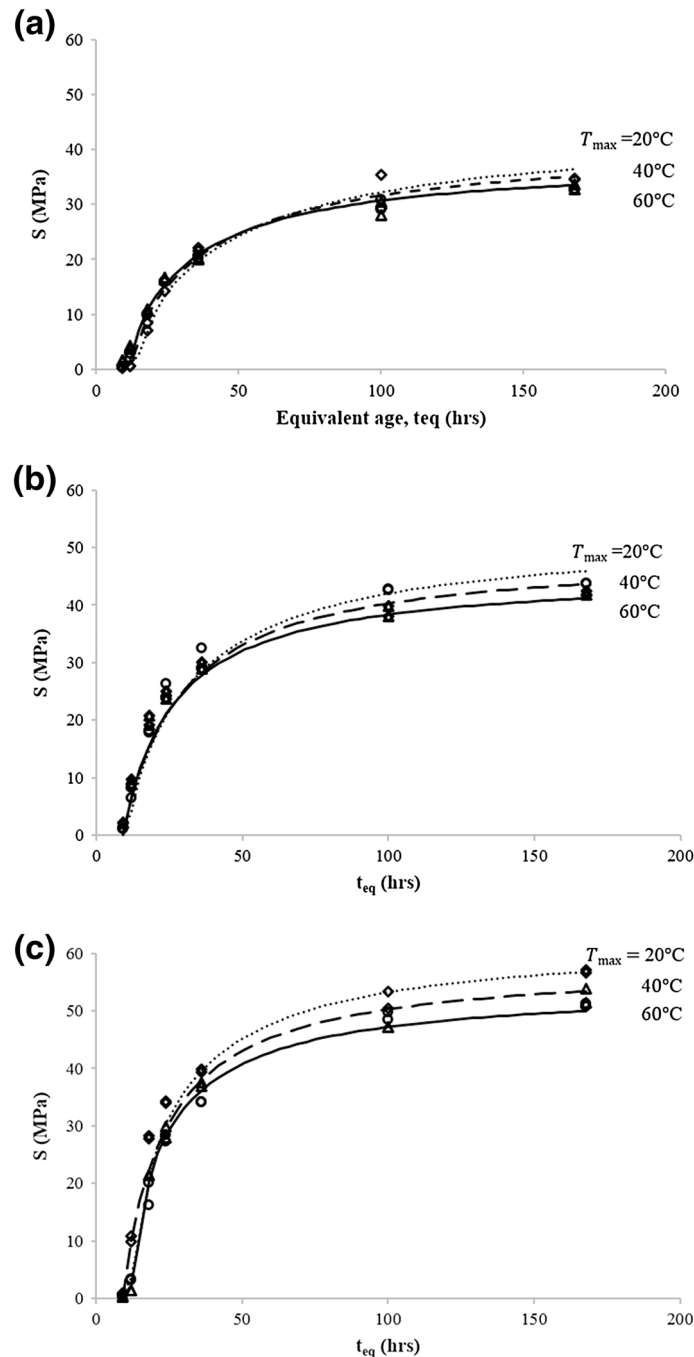


Fig. 12 Predictions by Eq. (10) for different f'_{cd} : Case IV in Table 4 (diamond 20 °C; open triangle 40 °C; and open circle 60 °C): **a** $f'_{cd} = 30$ MPa; **b** $f'_{cd} = 40$ MPa; **c** $f'_{cd} = 50$ MPa.

$$S = S_u \cdot \frac{1}{1 - \beta} \cdot \frac{A \cdot e^{-\frac{E}{R \cdot (T_r + 273)}} \cdot (t_{eq} - t_{0r})}{1.0 + A \cdot e^{-\frac{E}{R \cdot (T_r + 273)}} \cdot (t_{eq} - t_{0r})} \quad (10)$$

Statistical parameters resulting from the predictions by Eq. (10) are listed in Case IV in Table 3. The μ and σ for the ratios of predicted values to experimentally observed values were 0.97 and 0.22, respectively. Comparisons between the predictions made by Eq. (10) and 210 test results are shown in Fig. 11a. In Fig. 11b, model predictions for 18 test results for t_{eq} less than or equal to 18 h are presented. Compared with Figs. 10a and 10b, Figs. 11a, and 11b show improved predictions made by (Eq. 10), especially for data values before 18 h in t_{eq} . The developed model was able to predict

the test results with reasonable accuracy for all values of t_{eq} . Figure 12 illustrates comparisons between model predictions and 210 test results for different f'_{cd} . The model could predict strength development of HESC for all f'_{cd} and T_{max} values in a reasonable accuracy. The developed model can be used to find the optimum steam curing regime applied to HESC in future studies.

4. Conclusions

Application of HESC to steam-cured fabrication of precast prestressed units could reduce fuel consumption and carbon

dioxide emissions due to its early-strength-development. In order to maximize the beneficial effect of HESCS in steam curing process, it seems necessary to have a priori knowledge on the effects of curing temperature, age and other factors on the development of compressive strength in HESC. From experimental investigations, the following conclusions were made in this study.

1. HESC showed a general trend in strength development similar to that observed for normal concrete with Type I cement: a rapid increase during the early stage of curing followed by a gradual decrease in the rate in the later stage, with an asymptotic approach to the limiting compressive strength in the final stage.
2. Crossover effects were observed. Strength increased more rapidly at higher T_{max} in the early stage of curing but its rate reduced more rapidly in the later stage. This resulted in an asymptotic approach of strength to a relatively lower limiting compressive strength than that obtained under a lower T_{max} .
3. A majority of HESCs, regardless of differences in f'_{cd} and T_{max} , reached about 70 and 100 % of their design compressive strengths in 36 and 168 h in equivalent age, respectively.
4. Separate regression analyses on 3 sets of 70 data for each f'_{cd} exhibited that values of t_{0r} are independent of f'_{cd} or T_{max} and remain almost constant. However, values of S_u and k_r showed that they needed to be functionalized with influential factors.
5. From additional separate regression analyses performed for 15 sets of data, the values of A and t_{0r} were identified. Using these values, S_u and E were functionalized as a decreasing function with decreasing f'_{cd} and increasing T_{max} , and a decreasing function with increasing T_{max} and decreasing W/C , respectively.
6. A generalized model was presented after introducing a corrective function to better predict the strength development of HESC at an equivalent age of less than 18 h. The μ and σ for the ratios of predicted values by the generalized model to 210 experimentally measured data were 0.97 and 0.22, respectively.

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