

Effects of inlet momentum and orientation on the hydraulic performance of water storage tanks

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Abstract The influence of inlet momentum and inlet orientation on hydraulic performance of cylindrical water process tanks were investigated using a factorial design strategy. The hydraulic performance of the tanks was assessed with a computational fluid dynamics (CFD) model, which calculated the flow fields and the residence time distribution (RTD). RTDs were used to quantify the tanks hydraulic performance using hydraulic indexes that represent short-circuiting, mixing, and moment. These indexes were later associated with the effluent fraction of disinfectant (inlet and outlet disinfectant ratio). For small depth-to-diameter ratios, the inlet orientation and the inlet momentum were the most important factors regarding the hydraulic indexes and the effluent fraction of disinfectant, respectively. A poor correlation was obtained between the hydraulic indexes and the effluent fraction of disinfectant, indicating that they are not good predictors for water quality. For large depth-to-diameter ratios, the inlet orientation had the most significant effect on both the hydraulic indexes and effluent fraction of disinfectant. The short-circuiting and mixing indexes presented a good correlation with water quality for this case.

Keywords Numerical simulations · Computational fluid dynamics (CFD) · Residence time distribution (RTD) · Water quality · Factorial design

List of symbols

C	Tracer concentration (kg m^{-3})
C_o	Average tracer concentration (kg m^{-3})
$C'(\theta)$	Dimensionless RTD function
CFD	Computational fluid dynamics
D	Tank diameter (m)
d	Inlet diameter (m)
D_f	Molecular diffusivity ($\text{m}^2 \text{s}^{-1}$)
D_t	Eddy diffusivity ($\text{m}^2 \text{s}^{-1}$)
H	Water depth (m)
IO	Inlet orientation
k	Rate constant (day^{-1})
M	Inlet jet momentum ($\text{m}^4 \text{s}^{-2}$)
M_0	Zeroth moment of the dimensionless RTD function about the origin
M_1	First moment of the dimensionless RTD function about the origin
M_2	Second moment of the dimensionless RTD function about the origin
MI	Moment index
M_t	Tracer mass (kg)
Q	System volumetric flow rate ($\text{m}^3 \text{s}^{-1}$)
RTD	Residence time distribution
Sc	Turbulent Schmidt number
t	Dimensional time (s)
U	Velocity (m s^{-1})
U_{inlet}	Inlet velocity (m s^{-1})
V	Volume of fluid in the system (m^3)
X	Fraction of pollutant remaining over time for first-order reactions
τ	Theoretical residence time (s)
ν_t	Eddy viscosity coefficient ($\text{m}^2 \text{s}^{-1}$)
θ	Dimensionless time
θ_{10}	Dimensionless time necessary for 10 % of the tracer to leave the system

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Introduction

Treated water storage tanks are very common structures inside a water supply system, being usually designed and operated to meet hydraulic requirements such as: ensure the reliability of supply, maintain pressure, equalize pumping and treatment rates and improve operational flexibility and efficiency (NBR 12217 1994; AWWA 2002; Walski 2000). From a water quality standpoint, however, the impact of these tanks is often negative. The longer the water remains in a storage tank, the amount of disinfectant removed becomes greater. This can promote microbial regrowth in the distribution system, leading to taste and odor problems and increased survivability of human pathogens; it can also enhance the growth of harmful disinfectant by-products (Rossman and Grayman 1999; United States Environmental Protection Agency 2002). Therefore, design and operation guidelines should aim to minimize the water residence time in storage tanks to improve water quality and mitigate water disinfection costs.

In practice, each parcel of water may have a unique residence time affected by physical properties (e.g., water density), flow type (e.g., inlet momentum) and geometric parameters (e.g., water depth) of a tank. Therefore, estimation of residence time distribution (RTD) of each parcel of water is important, because they are used to calculate expected hydraulic and water quality efficiencies for specific designs. Although a number of field (Palau et al. 2007; Grayman 2000; Stamou 2002), laboratorial (Rossman and Grayman 1999; Tian and Roberts 2008; Manjula et al. 2010), and computational (Patwardhan 2002; Stamou 2002; Marek et al. 2007; Raja et al. 2007, 2008; Stamou 2008; Zhang et al. 2012, 2013, 2014; Xavier et al. 2014) experiments have been carried out to investigate the influence of design and operation factors upon hydraulic and water quality indexes, they are inconclusive on at least four issues.

Firstly, there is very little information available regarding the effect of design factors on hydraulic and water quality indicators other than mixing time (the time needed for a known amount of added tracer material to reach a specific degree of uniformity in the tank) (Kalai-chelvi et al. 2007). Mixing time is an indicator appropriate for measuring the performance of a storage tank if it is viewed as a mixing device. However, it is not clear if it can be expected all the water to mix before some of it leave the tank due to the enormity of their size (Van der Walt 2002). Therefore, although some experiments have been carried out in water storage tanks, there is very little information available about the effect of physical properties, flow type, and geometric parameters upon hydraulic and water quality indexes other than mixing time.

Secondly, to the authors knowledge, all studies used a poor experimental design, namely a one-factor-at-a-time strategy. In this strategy, design factors (e.g., inlet momentum, inlet orientation, and depth-to-diameter ratio) are analyzed by changing one factor at a time while holding the rest constant. However, a thorough investigation of the effects of the factors, along with their mutual interaction, is desirable for a better understanding of the subject. For example, Rossman and Grayman (1999) found that the main effect of inlet orientation (vertical or horizontal) is nearly zero. Therefore, we would be tempted to conclude that there is no effect of inlet orientation upon mixing time. However, the effect of inlet orientation may depend on the levels of another factor such as inlet momentum. If this is the case, then knowledge of the interaction between inlet orientation and inlet momentum is more useful than the knowledge of the main effect of inlet orientation. A significant interaction can mask the significance of main effects (Montgomery and Runger 2003). Consequently, Rossman and Grayman (1999) could have observed that the main effect of the inlet orientation may not have much meaning if a strong interaction is present. A factorial design strategy is the only way to compare these interactions (Brown and Berthouex 2010). One of the main advantages of this strategy, which is not possible in any one-factor-at-a-time strategy, is the possibility to estimate not only the main effects, but also all two-factor interactions and all higher-order interactions. In addition, the relative importance of all the factors can be evaluated simultaneously with a fewer number of experiments. Hence, the factorial design strategy can produce new and useful information.

Thirdly, many studies have been performed in physical models, which are not able to correctly reproduce both the turbulence and the decay rates of disinfectant and are unable to reproduce values of all the non-dimensional groups (e.g., Froude and Reynolds numbers) pertaining to the full-scale water storage tank (Shivaram 2007). These are particularly relevant in the study of the effect of physical properties, flow type, and geometric parameters upon hydraulic and water quality efficiency.

Finally, most of the studies used poor experimental techniques, i.e., invasive techniques with low spatial resolution (usually a few point-probe measurements in the tank) and high uncertainty that reveal little about the complex tracer concentration and velocity fields inside the tanks (Tian and Roberts 2008). Recently, Roberts et al. (2006) used a laser-induced fluorescence (LIF) technique to measure the temporal and spatial variations of tracer concentrations in water storage tanks, yielding far more detailed information than point-probe techniques. However, as Shivaram (2007) mentioned, the scanning planes

of Roberts et al. (2006) did not capture the increase in water height for the fill-and-draw tank operation model and therefore mixing time is underestimated.

In order to address these issues, computational experiments were conducted using a computational fluid dynamics (CFD) tool that allows obtainment of the flow field and RTD curves. Simulations were conducted on two full-scale tank types (depth-to-diameter ratios) with different inlet orientations and inlet jet momentum. The goal was to determine the empirical effects of known design factors (inlet momentum and inlet orientation) on hydraulic performance indicators using a factorial design. The hydraulic indexes were related to the water quality.

Residence time distribution (RTD) and Hydraulic performance indicators

The analysis of residence time distribution (RTD) functions originating from tracer studies is one of the main tools for the assessment of hydraulic performance in water storage tanks. RTDs can be obtained by an instantaneous injection of a known quantity of tracer mass, M_t , at the inlet section of the system and the subsequent measuring of the tracer concentration, C , along time, t , at its outlet section. In order to allow direct comparison of measured RTDs having dissimilar conditions (e.g., different volumes, flow rates, and tracer mass), they are usually presented in its normalized form (Werner and Kadlec 1996; Wahl et al. 2010). The dimensionless RTD function, $C'(\theta)$, and the dimensionless time, θ , can be defined, respectively, as:

$$C'(\theta) = \frac{C(\theta)V}{M_t} \tau \tag{1}$$

$$\tau = \frac{V}{Q} \tag{2}$$

$$\theta = \frac{tQ}{V} \tag{3}$$

where V is the system volume, Q is the system volumetric flow rate and τ is the theoretical (or nominal) residence time. The zeroth (M_0), first (M_1), and second (M_2) moment of the dimensionless RTD function about the origin are defined as, respectively:

$$M_0 = \int_0^\infty C'(\theta)d\theta \tag{4}$$

$$M_1 = \int_0^\infty \theta C'(\theta)d\theta \tag{5}$$

$$M_2 = \int_0^\infty (\theta - M_1)^2 C'(\theta)d\theta \tag{6}$$

The zeroth moment provides the fraction of tracer mass recovered. If the inlet tracer mass is substituted by the outlet tracer mass, M_0 is always equal to unity. When $M_0 = 1$, the first moment is the centroid of the RTD. The second moment is the variance of the RTD, which accounts for the spread of the tracer over time.

Certain RTD characteristics can be used as water storage tank performance indicators (Van der Walt 2002). Although the performance indicators focus on the hydraulic efficiency, they should also mirror the expected water quality. In the present work, three indicators are used, which are grouped into three broad categories: (1) short circuit, (2) mixing, (3) moment. The short-circuit indicator is taken to be θ_{10} , which is the time necessary for 10 % of the tracer mass to leave the tank. As a mixing indicator, M_2 was used. The indexes chosen for categories (1) and (2) were suggested by Teixeira and Siqueira (2008) after evaluating different short-circuit and mixing indexes under three criteria: the correlation of the index to the physical phenomenon it is said to represent; the capability of the index to detect variation; and statistical variability of the index. The moment index (category 3) provides a hydraulic efficiency measurement that avoids reliance on short-circuiting and mixing indexes. In this study, the post-nominal moment component, suggested by Wahl et al. (2010), was adopted. This approach considers hydraulic efficiency relative to the fraction of tracer exiting later as well as the juxtaposition of residence times about what is referred to as the nominal divisor in Fig. 1. This method assumes that the residence times of a completely efficient water storage tank will meet or fall to the right of the nominal divide. The portion of tracer exiting the storage tank later than the nominal divide adversely impacts water quality. The segment of the probability density function to the right of the nominal divide is considered inefficient, with more weight assigned to the more severely overdue

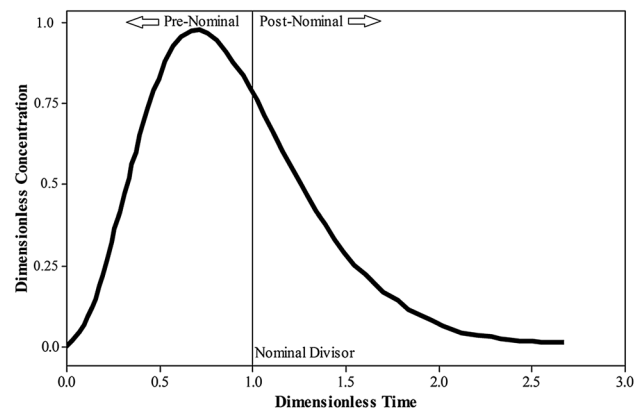


Fig. 1 Residence time distribution showing pre-nominal and post-nominal components. Figure adapted from Wahl et al. (2010)

residence times. If the bulk of tracer exiting has a close proximity to the nominal divide, then hydraulic efficiency is high. As more tracer exits earlier (in relation to the nominal divide), the hydraulic efficiency approaches zero. The post-nominal moment index is equal to:

$$MI = \int_1^{\infty} (\theta - 1)C'(\theta)d\theta \quad (7)$$

Materials and methods

Factorial design strategy for the CFD experiments

A factorial design strategy was used to study the effects of all combinations of inlet momentum and orientation on four responses for two water-depth-to-tank-diameter ratios. The response variables for observing the hydraulic and water quality performance were θ_{10} , M_2 , MI, and X (see definition of X in Eq. 9). The two factors inlet momentum and inlet orientation were each investigated at two levels. This is a two-level, two-factor experimental design. Two factors at two levels gives four experimental conditions for each H/D (Tables 1, 2; Fig. 2). For $H/D = 0.25$, four experimental conditions were added to the two-level factorial design to obtain an independent estimate of error and to check for curvature. For $H/D = 4$, the values considered for M were $9.63 \times 10^{-5} \text{ m}^4/\text{s}^2$ (low level) and $1.38 \times 10^{-4} \text{ m}^4/\text{s}^2$ (high level). The inlet orientation was set between a horizontal

inlet, 10 cm above the bottom (low level); and a vertical inlet, 10 cm from the sidewall (high level). For $H/D = 0.25$, the values considered for M were $1.23 \times 10^{-3} \text{ m}^4/\text{s}^2$ and $4.93 \times 10^{-1} \text{ m}^4/\text{s}^2$. The inlet orientation was set between a horizontal inlet, 10 cm above the bottom; and a vertical inlet, 10 cm from the sidewall. For both types of tanks, the outlet was located at the center of the bottom of the tank. The ranges of data tested were chosen based on characteristics found in typical Brazilian storage tanks; they were not intended to be representative of the entire range of field conditions, but rather intended to determine if the inlet momentum and inlet orientation had an effect on the responses. The effects of the factors of inlet momentum and inlet orientation were evaluated by determining the significance of main factors (main effect) and their interaction using Lenth's pseudo-standard error (PSE). The pseudo-standard error is based on the concept of sparse effects, which assumes the variation in the smallest effects is because of random error (MINITAB 2015). The main effect is defined as the change in response produced by a change in the level of the factor. The interaction between the factors examines the effects of inlet momentum at different levels of inlet orientation. Factorial design strategies are the only way to discover interactions between factors.

CFD

Initially the flow field was determined through 3D steady state simulations by solving the Reynolds averaged Navier–

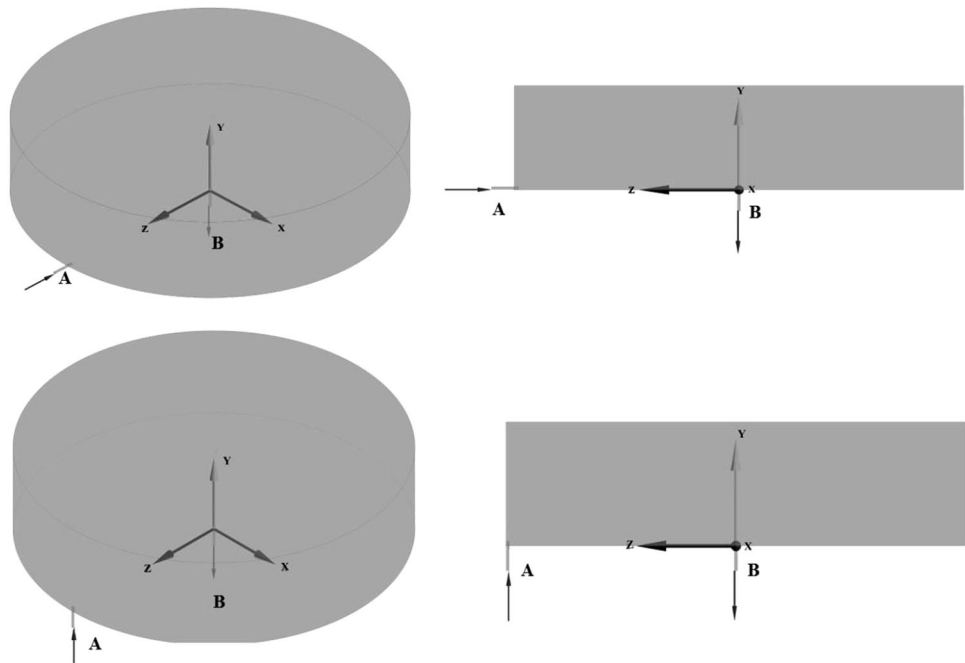
Table 1 Design matrix of the 2^2 factorial experimental design (with extra points), levels of independent variables (M and inlet orientation) and observed responses (θ_{10} , M_2 , MI, X) for $H/D = 0.25$

Experiment	d (m)	τ (day)	M (m^4/s^2)	Inlet orientation	Reynolds number	θ_{10}	M_2	MI	X
1	0.15	3.33	1.77×10	Horizontal	4.75×104	0.014	0.590	0.217	0.452
2	0.07	3.33	7.09×10	Horizontal	9.50×104	0.052	0.687	0.309	0.392
3	0.35	0.083	4.93×10	Horizontal	7.92×105	0.008	1.122	0.375	0.965
4	0.18	3.33	1.23×10	Horizontal	3.96×104	0.001	0.602	0.236	0.471
5	0.15	3.33	1.77×10	Vertical	4.75×104	0.123	0.450	0.209	0.389
6	0.07	3.33	7.09×10	Vertical	9.50×104	0.131	0.614	0.173	0.358
7	0.35	0.083	4.93×10	Vertical	7.92×105	0.143	0.482	0.233	0.965
8	0.18	3.33	1.23×10	Vertical	3.96×104	0.116	0.448	0.205	0.395

Table 2 Design matrix of the 2^2 factorial experimental design, levels of independent variables (M and inlet orientation) and observed responses (θ_{10} , M_2 , MI, X) for $H/D = 4$

Experiment	d (m)	τ (day)	M (m^4/s^2)	Inlet orientation	Reynolds number	θ_{10}	M_2	MI	X
9	0.15	3.33	1.38×10^{-4}	Horizontal	1.33×104	0.018	0.673	0.250	0.490
10	0.07	3.33	9.63×10^{-5}	Horizontal	1.11×104	0.001	0.850	0.292	0.507
11	0.35	3.33	1.38×10^{-4}	Vertical	1.33×104	0.115	0.427	0.194	0.399
12	0.18	3.33	9.63×10^{-5}	Vertical	1.11×104	0.115	0.536	0.250	0.381

Fig. 2 3D view and plane $x = 0$ (plane z - y) of the geometry of the tanks used in the computational simulations where, A inlet; and B outlet. The origin of the axis is at the outlet pipe



stokes (RANS) equations via the k - ϵ turbulence model using the commercial computational fluid dynamics (CFD) code ANSYS CFX[®] 14.5.7. The steady-state simplification is justified for tanks that are not subjected to large tank level fluctuations (Van der Walt 2002). Pressure-velocity coupling was achieved by using the semi-implicit method for pressure-linked equations (SIMPLE) algorithm. A second order upwind scheme was used for spatial discretization of the flow equations. The advection scheme chosen, as well as the turbulence numeric, was the high resolution. Boundary conditions were defined at the borders of the computational domain. A uniform flow was imposed at the inlet. At the outlet, an average static reference pressure of 0 Pa was specified. A no-slip boundary condition was applied at the walls. The free surface was considered a symmetry plane, which implies a zero gradient for all variables normal to that plane (Stamou 2002). Hence, the free surface was assumed to be flat.

Once the steady-state flow field was obtained, the transport of a scalar quantity C was simulated by solving the three-dimensional Reynolds-averaged advection-diffusion equation:

$$\frac{\partial C}{\partial t} + U_j \frac{\partial C}{\partial x_j} = \frac{\partial}{\partial x_j} \left((D_f + D_t) \frac{\partial C}{\partial x_j} \right) \quad (8)$$

where t is the time, U is the velocity, D_f is the molecular diffusivity and $D_t (=v_t/Sc)$, v_t is eddy viscosity coefficient and Sc is the turbulent Schmidt number) is the eddy diffusivity. The turbulent Schmidt number, Sc , was 0.9. Equation 8 is based on the values of U_j , which are obtained from the converged steady-state hydrodynamics simulation.

A pulse tracer study was conducted in each tank. At the inlet, a passive and conservative tracer was injected having a duration less than 2 % of the nominal detention time and represented by a square step input. The scalar concentration was monitored at the outlet to produce the RTD curve for the tank. The tracer transport simulations ran until 95 % of the tracer mass left the tank. The solute transport simulations were carried out using a time step that varied between 1 and 600 s. For the time intervals where there was a high variation of concentration, the time step was smaller.

The full domain of the numerical grid for a typical simulation is shown in Fig. 3. The numerical code employs unstructured numerical grids, which permit a very accurate representation of the boundaries. The grid had a finer spacing at the inlet and outlet regions. After a series of preliminary calculations, the computational grids for the twelve cases ranged from 10^4 to 10^5 hexahedral elements. The preliminary results indicated that this grid was fine enough to capture the flow features that were important while providing a satisfactory computational time. More details of the governing equations, turbulence model, and algorithms can be found in the CFX[®] user's guide (ANSYS Inc. 2012).

Results and discussion

Validation

Prior to its application, the CFD model was validated experimentally by finding the RTD of Kipseli's tank in

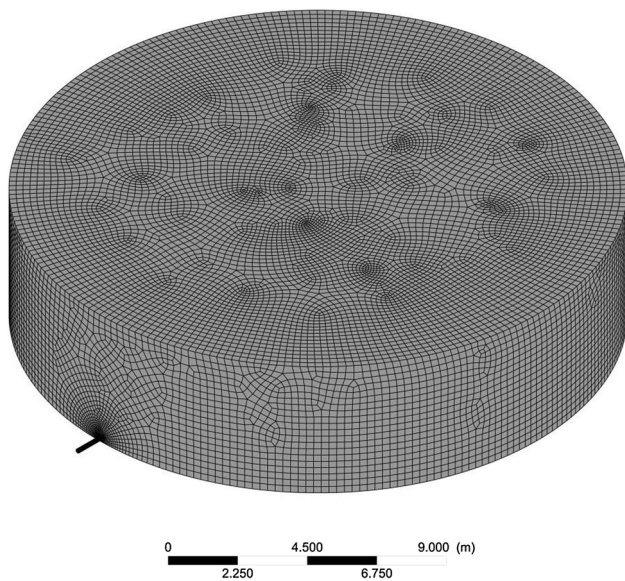


Fig. 3 Typical mesh used in the CFD simulations. It is possible to notice the refinement at the *inlet* region, where the velocity gradients are greater when compared to the rest of the tank

Athens, Greece (Stamou 2002), which has a horizontal section of 1560 m^2 . The water flows into the tank from a 600 mm inlet pipe at the bottom of the tank and exits via two 900 mm diameter pipes, placed in a hopper at the bottom of the tank. The 41 columns in the tank were disregarded to simplify the geometry. The nominal residence time is around 100 min. A pulse tracer study was conducted by injecting a mass of sodium chloride (inlet concentration $C_0 = 0.07 \text{ kg/m}^3$) for approximately 3 % of the nominal residence time into the inlet pipe of the tank. There is a good agreement between experimental and computational RTD data (Fig. 4), taking into consideration that only one RTD experiment has been performed and that Q and H were not constant during the field tracer experiment. At the first wave of the curve, the peak of the experimental curve is higher than both computational results. At the fifth and sixth waves of the curve the computational results displays a good agreement with the experimental data, as well as the later part of the curve, which has an exponential shape. For $t/\tau > 0.75$, both experimental data and computational results almost coincide. The region where the computational results present the least agreement with the experimental data is at the second, third and fourth waves, where the computational results presents a similar behavior, but with a shorter period. The period, as commented by Stamou (2002), is the time needed for the tracer to complete a passage in a large recirculation region, which occupies almost 95 % of the volume of the tank (figure not shown in the text). These shorter periods suggest that there is a higher level of mixing in most of the when compared to the CFD-model.

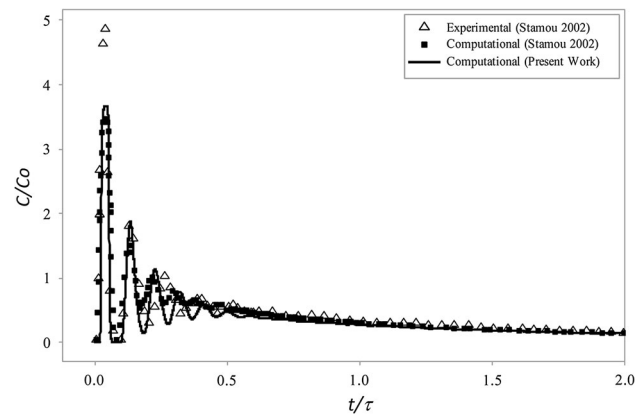


Fig. 4 Comparison between the experimental data and the computational results

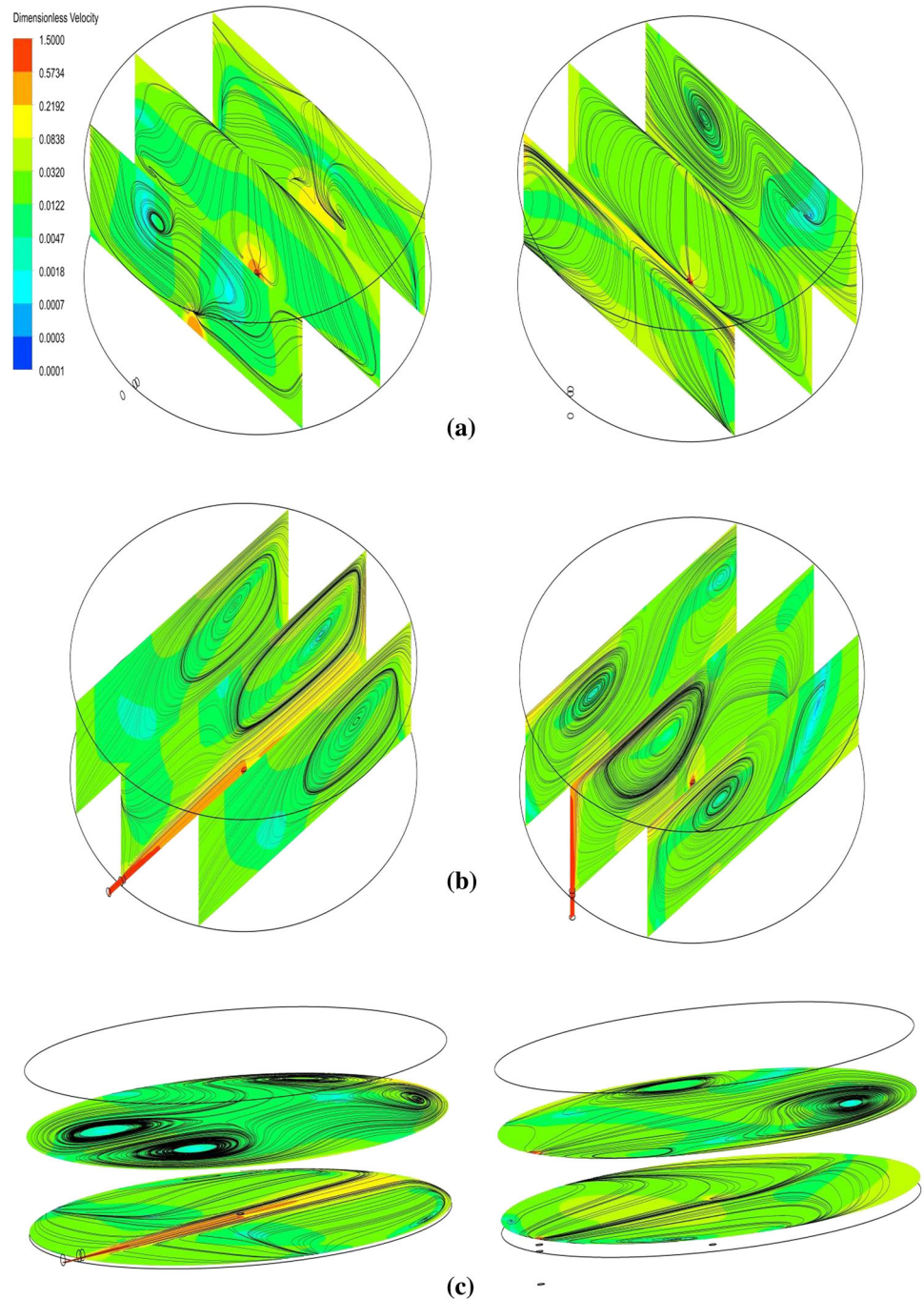
As suggested by Stamou (2002), these differences can be partly attributed to the additional turbulence created by the 41 columns in the tank, which are not taken into account for in the CFD-model. Hence, considering that the CFD-model is actually a simplification of a real tank geometry and operation, the resulting RTD exhibits a remarkable fidelity to the experimental RTD.

$$H/D = 0.25$$

Velocity field and streamlines

Simulated distributions of the velocity magnitude and streamlines in planes xy , yz , and xz are shown in Fig. 5 for $H/D = 0.25$. The velocity U was normalized by the inlet velocity, U_{inlet} . For the horizontal inlet (Fig. 5), the circular jet near the bottom of the tank evolves into a wall jet along the tank's bottom wall. As the inlet jet went toward the opposite wall with high velocity, part of the flow was diverted towards the tank outlet, so that significant volumes of water exit the tank in a much lower time than the theoretical residence time, τ . Despite using a different outlet configuration (the outlet was located on the floor opposite the inlet in the mid-plane) and depth-to-diameter ratio (H/D oscillated between 0.8 and 0.9), Zhang et al. (2014) also found that part of the influent flows to the outlet directly. When the jet impinged on the opposite wall (Fig. 5), the flow was re-directed from a horizontal to a vertical direction inducing counter-clockwise circulations that consist mainly of vertical two-dimensional circulation cells in the back half of the tank (opposite the inlet wall). This behavior is consistent with the vertical flow structure observed by Maruyama et al. (1982). The horizontal flow field is dominated by recirculation regions. High velocities were observed in the outer parts of the recirculation regions. Very low velocities were found in the central areas

Fig. 5 Isocontours of dimensionless velocities, U/U_{inlet} , and streamlines for $H/D = 0.25$. **a** xy planes ($z = -5, 0,$ and 5 m); **b** yz planes ($x = -5, 0,$ and 5 m); **c** xz planes ($y = 0.1$ and 3 m) The left tank has a horizontal inlet orientation while the right has a vertical inlet orientation



of the re-circulation regions. A similar horizontal flow pattern was also observed for our other horizontal inlet cases. Zhang et al. (2014) also reported the existence of recirculation regions for the horizontal flow field. When the inflow rate obtained by Zhang et al. (2014) was greater than the outflow rate, two big flow recirculation regions were found to occupy a large portion of the tank and two secondary flow recirculation regions were found near the outlet; when the inflow rate was less than the outflow rate, the big flow recirculation regions breaks into four.

For the vertical inlet (Fig. 5), flow enters the tank through the inlet as a jet and flows with high velocity towards the free surface. At the water surface, the upward flow was spread into one lobe towards the center and two circulation zones to the left and right of the central surface jet. As the water flows toward the center, it was diverted toward the tank bottom, creating a recirculation zone around an axis parallel to the x -axis. Some portion of this water gets back into the jet to mix with new water entering through the inlet. The other portion of water flows out

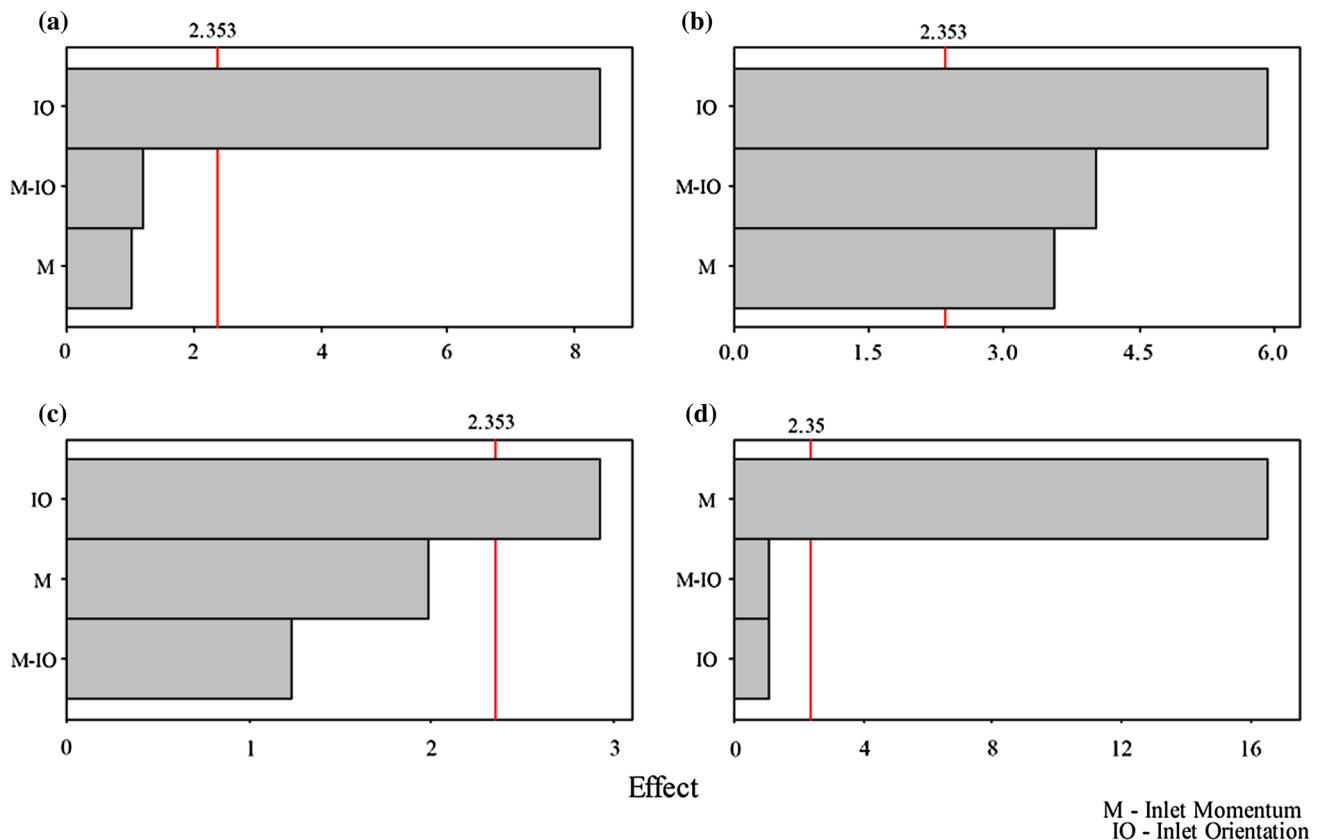


Fig. 6 Pareto charts of the effects for the 22 factorial design over: **a** θ_{10} ; **b** M_2 ; **c** MI; and **d** X; for $H/D = 0.25$

through the outlet. Again high velocities were found near the inlet, the outlet, and in the outer parts of the recirculation regions. Very low velocities were observed in the central areas of the re-circulation regions.

Tian and Roberts (2008) measured tracer concentration distributions using laser-induced fluorescence (LIF) for storage tanks with $H/D = 0.25$ and two values of M , and no outlet. They found results that are consistent with our simulations. The inflowing jet was a region of high concentration. Mixed inflow separated from the water surface, forming a recirculation zone that gets back into the jet. More mixed inflow appeared near the tank wall opposite the jet due to flow that travels around the tank walls. After it appears in the center plane, it moves back toward the jet and is re-entrained by it. With a single outlet of the same diameter near the opposite wall, Tian and Roberts (2008) observed reduced recirculation.

Hydraulics indicators

In order to better understand the hydrodynamics processes, hydraulic indexes were calculated for the $H/D = 0.25$ tanks (Table 1). The average of θ_{10} was low ($\theta_{10} = 0.0735$), indicating the presence of intense short-

circuiting. In general, short-circuiting is never desirable because it reduces the effective volume of the storage tank and, consequently, disinfectant residual loss is increased. θ_{10} increased by 0.120 as the inlet orientation changed from horizontal to vertical. When inlet momentum M increased, θ_{10} also slightly increased (0.018). The influence of the M and IO (Inlet Orientation) interaction on θ_{10} was low (0.017). To establish if these effects are significant, the pareto charts were constructed (Fig. 6). The Pareto chart displays the relative importance of the main and interaction effects. The vertical line in the chart indicates the minimum statistically significant effect magnitude for a 90 % confidence level. The interpretation of this chart demonstrated that the factor of inlet orientation is statistically significant for θ_{10} . On the contrary, the inlet momentum and the interaction between inlet momentum and inlet orientation were not statistically significant. In other words, a vertical inlet diminished the short-circuiting.

The average M_2 effect was 0.624. M_2 is a quantitative description of the tanks mixing scale. According to ideal reactor models, an M_2 value close to 0 indicates plug flow while a value close to 1 indicates that the flow is closer to the complete mixing regime (Rossman and Grayman 1999). Generally, mixed flow is assumed to result in less

disinfectant loss (Grayman 2000). If this is true, any $M_2 < 1$ increases the disinfectant loss. Mixing indicators ranged from a transitional flow (between mixed and plug) to a completely mixed tank (0.448 to 1.122). The change in the inlet position from horizontal to vertical decreased M_2 , while increasing the inlet momentum from low to high increased its value. The interaction M -IO was equal to 0.259, i.e., the effect of inlet momentum was dependent on the level of inlet orientation. The effect of inlet orientation upon mixing was much stronger for a high inlet momentum than for a low inlet momentum. The sequence of the most important effects with respect to M_2 was found to be $IO > M$ -IO $> M$. All main effects and interactions were statistically significant.

The average MI was 0.245. Since the MI values were close to zero (MI = 0.17–0.37), the hydraulic efficiency is good. Inlet orientation plays a major role upon MI. Change in inlet position from horizontal to vertical decreased MI by 0.1. The inlet momentum and the interaction between inlet momentum and inlet orientation were not statistically significant.

For first-order reactions, it is possible to determine the fraction of disinfectant (X) remaining over time depending on some rate constant k as follows (Wahl et al. 2010):

$$X = \int_0^{\infty} \frac{C'(\theta)}{\tau} e^{-kt} dt \tag{9}$$

Considering $k = 0.5 \text{ day}^{-1}$ (Grayman 2000), Table 1 presents the values for X . The average X was 0.550. The fraction of disinfectant varied from 0.36 to 0.96. X increased with the increase of M . Increasing M from 1.23×10^{-3} to $4.93 \times 10^{-1} \text{ m}^4/\text{s}^2$ increased X by a maximum of 55 %. The main effect of inlet orientation and its interaction with the inlet momentum were not statistically significant for X .

In summary, the strongest influence on the hydraulic efficiency was the inlet orientation. The horizontal inlet orientation resulted in the best hydraulic efficiency. Previous studies considering mixing time have also observed the influence of inlet orientation on hydraulic efficiency (Maruyama et al. 1982; Dakshinamoorthy et al. 2006; Zughbi and Rakib 2004; Tian and Roberts 2008). However, contradictory results showed up about how the inlet orientation influences the hydraulic efficiency (Shivaram 2007). One of the reasons they show contradictory results might just come down to experimental design. Inlet momentum and the interaction between inlet momentum and inlet orientation also had influence on the hydraulic efficiency, particularly for the mixing index. The effect of inlet orientation upon mixing was much stronger for a high inlet momentum than for a low inlet momentum. In general, similar results have been reported in laboratory

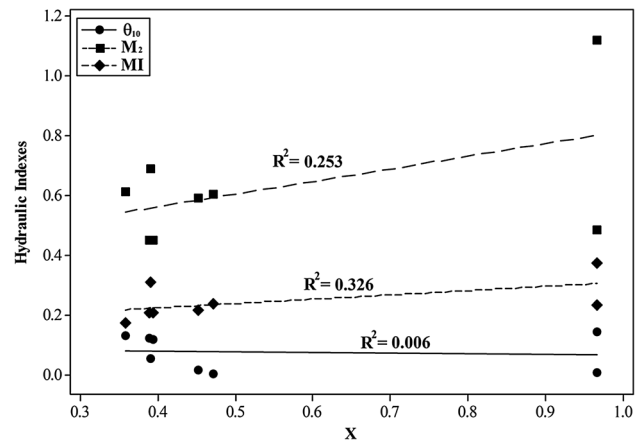


Fig. 7 Hydraulic indices plotted vs. effluent disinfectant fraction, X , for $H/D = 0.25$. Regression lines are also displayed along the data displayed with R^2 values for θ_{10} , M_2 , and MI

Table 3 Average and main effects of M and IO and their higher order interactions of the 2^2 factorial design on the θ_{10} , M_2 , MI, and X for $H/D = 0.25$

Effect	θ_{10}	M_2	MI	X
Average effect	0.0735	0.624	0.245	0.550
Main effects				
M	+0.018	+0.278	+0.083	+0.531
IO	+0.120	-0.379	-0.100	-0.029
Two-factor interactions				
$M - IO$	+0.017	-0.259	-0.042	+0.029

experiments (Tian and Roberts 2008). All these studies, however, evaluated the hydraulic efficiency using the mixing time indicator and they did not affirm the relative importance of inlet orientation, inlet momentum and their interaction.

Regarding water quality efficiency, the strongest influence was the inlet momentum. Since the order of importance of the factors that influence the hydraulic efficiency were different from those that influence the water quality efficiency, the hydraulic indexes should not demonstrate good correlation to the effluent disinfectant fraction X . This is confirmed by the low correlation between the hydraulic indexes and X (see Fig. 7). Therefore, judging the water quality of a small depth-to-diameter ratio storage tank, based on traditional hydraulic indexes, should be avoided (Table 3).

$H/D = 4$

Velocity field and streamlines

Simulated distributions of velocity magnitude and streamlines in the planes xy , yz , and xz are shown in Fig. 8 for $H/$

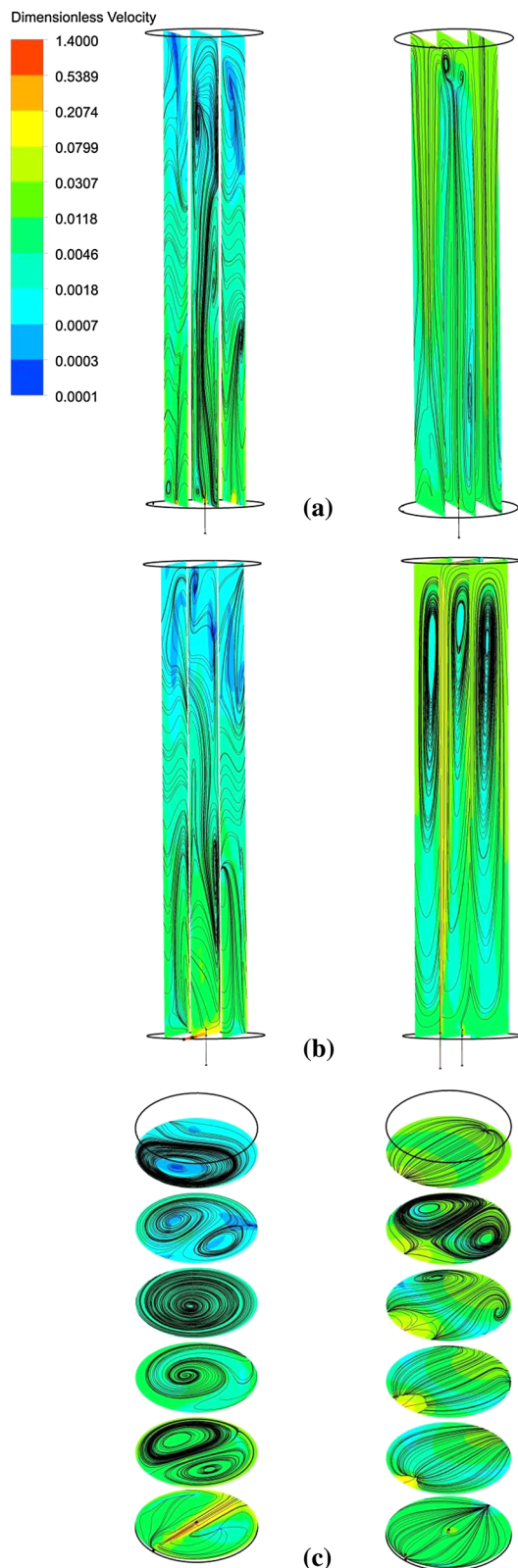


Fig. 8 Isocontours of dimensionless velocities, U/U_{inlet} , and streamlines for $H/D = 4$. **a** xy planes ($z = -1, 0, \text{ and } 1 \text{ m}$); **b** yz planes ($x = -1, 0, \text{ and } 1 \text{ m}$); **c** xz planes ($y = 0.1, 3, 6, 9, 12, \text{ and } 15 \text{ m}$). The left tank has a horizontal inlet orientation while the tank to the right has a vertical inlet orientation

in the tanks with $H/D = 0.25$, with the presence of a short-circuit zone governed by the inlet jet. When the jet hits the opposite wall and heads to the upper parts of the tank it loses energy, diminishing the magnitude of the velocities. The velocity continues to decrease as the water moves up. This can result in very different degrees of mixing between the lower and higher regions. Tian and Roberts (2008) using LIF measurements observed a similar behavior for tanks with $H/D = 2.5$. They found that the flow separates into two regions, with higher concentrations in the lower region, and lower concentrations in the upper region. Mixing between these two regions was slow, resulting in longer mixing times. Furthermore, there is a tendency for two recirculation zones (xz -planes) to appear at lower regions of the tank (these recirculation zones are divided by the inlet jet). In the higher regions, these zones tend to merge and form a larger recirculation zone at the upper part of the tank (where the inlet jet influence is less significant). For the vertical inlet, it is possible to observe that the magnitudes of the velocities are more uniform throughout the tank. A big recirculation zone can be observed around an axis parallel to the x axis. High velocities were found near the inlet, the outlet, and in the outer parts of the recirculation regions. Very low velocities were observed in the central areas of the re-circulation regions.

Hydraulic indicators

Table 4 presents the main effects of M and inlet orientation and their interactions on θ_{10} , M_2 , MI, and X for $H/D = 4$. Figure 9 shows the pareto chart of the effects of the 2^2 factorial design over θ_{10} , M_2 , MI, and X for $H/D = 4$. The values of θ_{10} were low (the average is equal to 0.0622), indicating the presence of intense short-circuiting. The short-circuiting average time was shorter for $H/D = 4$ in comparison to $H/D = 0.25$. The effect of the inlet orientation was statistically significant and θ_{10} increases by 0.110 as the inlet orientation changed from horizontal to vertical. Inlet momentum and its interaction with the inlet orientation did not present a statistically significant contribution.

With respect to M_2 , the average is 0.621. This is almost the same average value than that observed for $H/D = 0.25$. The mixing index was in the range of 0.427–0.850 (Table 2), i.e., a flow regime between transitional (between mixed and plug) and completely mixed. The pareto chart shows that the main effects of M and IO and their

$D = 4$. Again, U/U_{inlet} was used to analyze the flow patterns of the tanks. For the horizontal inlet, the behavior at the lower regions of the tank was similar to the one found

Table 4 Average and main effects of M and IO and their higher order interactions of the 22 factorial design on the θ_{10} , M_2 , MI , and X for $H/D = 4$

Effect	θ_{10}	M_2	MI	X
Average effect	0.0622	0.621	0.250	0.444
Main effects				
M	+0.008	-0.143	-0.050	+0.001
IO	+0.110	-0.280	-0.050	-0.108
Two-factor interactions				
$M - IO$	-0.009	+0.034	-0.007	+0.017

interaction do not significantly affect the mixing indicator M_2 (Fig. 9).

Regarding MI , the average value was 0.250, not so different from the average value of 0.245 noted for $H/D = 0.25$. The MI values are in the range of 0.194–0.292 (Table 2). All main effects and interactions were insignificant for MI (Fig. 9).

Again, considering $k = 0.5 \text{ day}^{-1}$ (Grayman 2000). The fraction of disinfectant varied from 0.381 to 0.507 (Table 2). The average X was 0.444. Compared to the $H/D = 0.25$ tanks, the $H/D = 4$ tanks reduced the average water quality by about 19 %. X decreased when inlet

orientation changed from horizontal to vertical by a maximum of 21.7 %. The main effect of inlet momentum and its interaction with inlet orientation were not statistically significant for X .

In summary, the strongest influence on the hydraulic efficiency was, once again, the inlet orientation. The effects of inlet momentum and the inlet momentum-inlet orientation interaction appeared small (statistically insignificant). Considering the effect of the inlet orientation, similar results have been reported in laboratory experiments (Tian and Roberts 2008). These studies, however, as mentioned before, evaluated hydraulic efficiency using the mixing time indicator and they did not affirm the relative importance of inlet orientation, inlet momentum and their interaction. Regarding the water quality efficiency, the strongest influence was also the inlet orientation. Since the strongest influence on hydraulic and water quality efficiencies is the same, at least one hydraulic index should demonstrate good correlation to effluent disinfectant fraction X . This is confirmed by the R^2 value of 0.787 for the mixing index and the R^2 value of 0.986 for the short-circuiting index (Fig. 10). Again, the moment index does not demonstrate good correlation with X . Therefore, the use of short-circuiting and mixing indexes could be simple indicators of the water quality in storage tanks with $H/D = 4$.

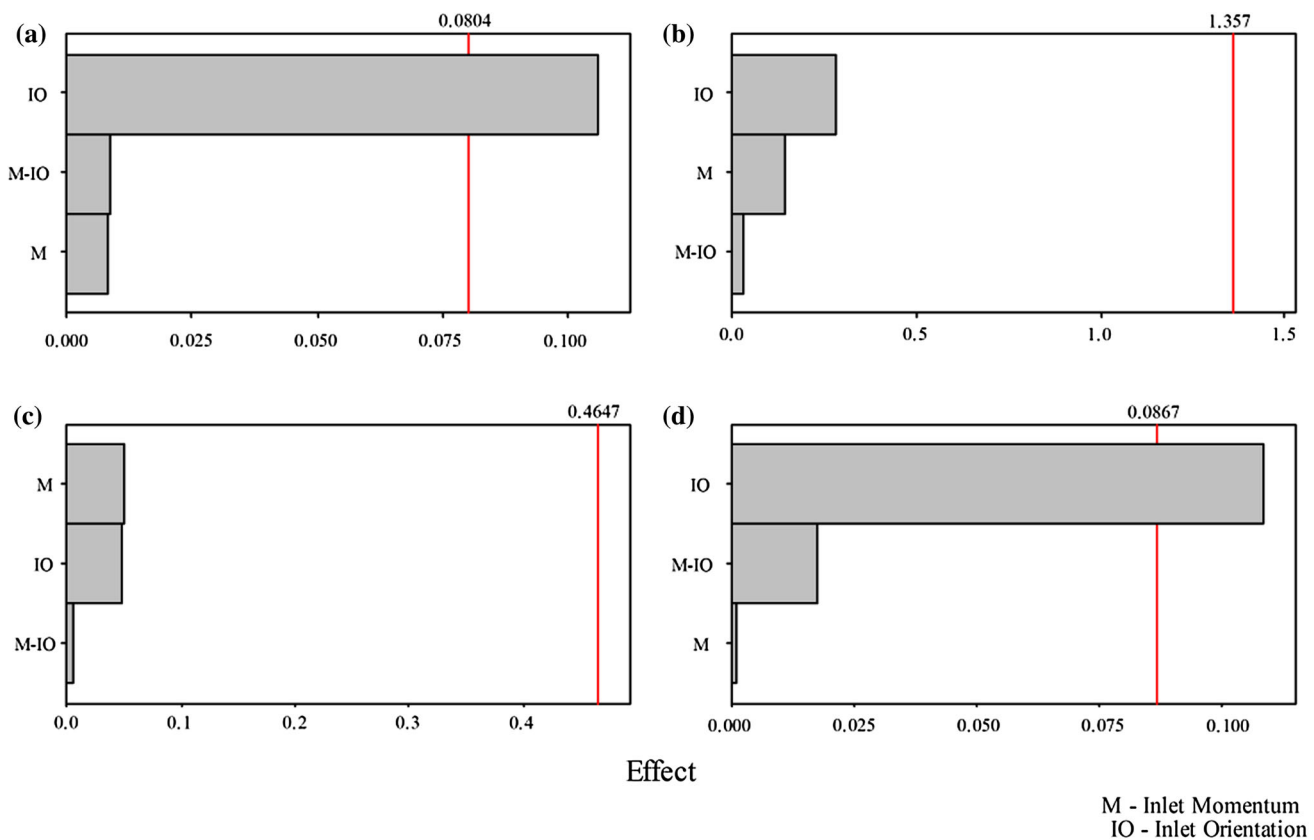


Fig. 9 Pareto chart of the effects of the 22 factorial design over: **a** θ_{10} ; **b** M_2 ; **c** MI ; and **d** X ; for $H/D = 4$

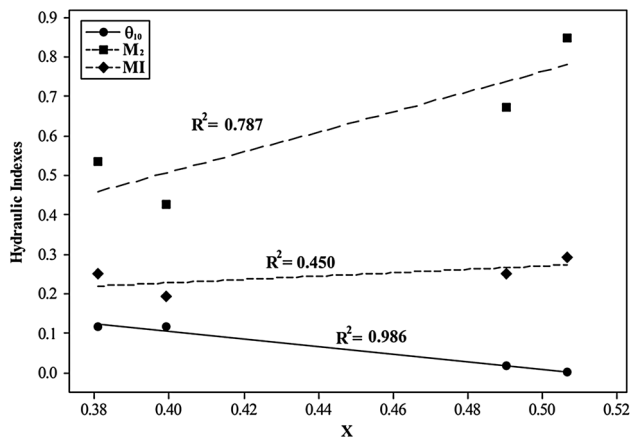


Fig. 10 Hydraulic indices plotted vs. effluent disinfectant fraction, X , for $H/D = 4$. Regression lines are displayed with R^2 values for θ_{10} , M_2 , and MI

Conclusions

CFD was applied to determine the empirical effects of inlet momentum and inlet orientation upon the hydraulic efficiency and the water quality of water storage tanks using a factorial design. For a small depth-to-diameter ratio ($H/D = 0.25$), the most important factor for hydraulic efficiency was the inlet orientation. Inlet momentum and the interaction between inlet momentum and inlet orientation also had an influence on the hydraulic efficiency. The effect of inlet orientation upon mixing is much stronger for a high inlet momentum than for a low inlet momentum. Regarding the water quality efficiency, the strongest influence was the inlet momentum. Inlet orientation and the interaction inlet orientation-inlet momentum are not significant. Since the order of importance of the factors that influence the hydraulic efficiency vary from those that influence the water quality efficiency, none of the hydraulics indexes demonstrated good correlation to water quality. For large depth-to-diameter ratio ($H/D = 4$), the inlet orientation had the strongest influence on both the hydraulic efficiency and the water quality. None of the other factors were significant. The short-circuiting and mixing indexes presented a good correlation with the effluent disinfectant fraction and could be used to indicate the water quality for large depth-to-diameter ratio storage tanks.

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