




# Photogrammetric evaluation of shear modulus of glulam timber using torsion test method and dual stereo vision system

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## Abstract

The shear modulus of timber and timber-based composite materials is a fundamental mechanical property, which is used in the design of timber and engineered wood products. The problem of experimentally determining appropriate values of shear modulus for timber-based composite is not as simple and straightforward as in isotropic materials. Although the torsion test is a recommended standard approach to determine the shear modulus of structural-size timber and glulam beams, it is difficult to measure the rotational deformations of the timber beams. Therefore, in this paper, a stereo camera system combined with a photogrammetric approach is proposed to evaluate the values and variations of the shear modulus of glulam beams under the torsion test. The photogrammetric approach is a non-contact method, which provides an efficient and alternative approach for measuring the deformations of the torsion specimens in three dimensions. A series of experiments was conducted on glulam timber beams under the torsion test to investigate the applicability of the optical approach to evaluate the values and variations of shear modulus as well as to investigate the effect of applying torques in a clockwise or anticlockwise direction on the shear modulus of the beams. This optical system not only allows the performance and reliability of the traditional sensors to be assessed, but also allows the rotational deformation of the torsion samples to be monitored at various locations. This enables the values of shear modulus at different cross-sections of the torsion specimens to be evaluated without the need to use more devices. The test results showed that applying torques to the glulam timber specimens during loading and unloading in either a clockwise or anticlockwise direction does not influence or cause a significant change in the shear modulus of the beams. By comparing shear modulus values of glulam beams measured based on different shear spans, it was found that the larger the shear span the smaller the shear modulus value. This might indicate that the variations of shear modulus values at these different gauge lengths need to be considered.

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## 1 Introduction

The shear modulus of produced timber and timber-based composite beams is of considerable importance as it plays a very significant role in structural modelling, such as the lateral torsional stability of beams (EN 1995-1-1 2004), and designing serviceability of wood-beam floors (Foschi 1982). It is also an important input for developing analytical and finite element models (Chui 2002; Jiang et al. 2004). The torsion test provides a state of pure shear stress distribution in the specimen (Hindman et al. 2005a; Gharavi et al. 2018, 2017; Gharavi and Zhang 2018), allowing measurement of the pure shear properties. This method is considered to be a suitable approach for determining the shear modulus of timber and timber-based composite materials. The most recently updated British Standard, BS EN 408:2010+A1:2012 (2012) recommended the torsion test as a standard approach to

determine the shear modulus of structural-size timber and glulam beams. Recently, the torsion test has been adopted by more and more researchers (including Gupta et al. 2002; Gupta and Siller 2005; Hindman et al. 2005a, b; Khokhar et al. 2010; Zhang et al. 2011, 2012) to evaluate the shear modulus of structural-size timber and glulam beams.

In the torsion test, the exact measurement of the relative angle of twist over a gauge length is essential to determine the shear strain and hence enable the calculation of shear modulus. The rotations measured by the torsion tester machines, such as the Tinius Olsen machine, may include small slippage between the board and the clamps of the testing machine. To overcome this difficulty, angle measurement devices, such as inclinometers, are traditionally used to measure the rotations of the torsion specimens. Although this mechanical device has proven to be accurate and flexible for measuring the rotations, it presents some significant drawbacks. Configuring the sensor tends to be time-consuming and labour intensive and it can be damaged or destroyed during destructive testing. The size of this sensor and the nature of the surface contact it is measuring have prevented it from being used to measure more details of the shear deformations, details which might help to evaluate the variations of shear modulus ( $G$ ) of timber and glulam beams. The photogrammetric approach is proposed in this research to measure the surface rotations of the torsion samples.

Monitoring tools using photogrammetry and image processing have been used in timber to measure mechanical properties, 3D displacements, or deformations at various points of the same specimen, as proposed in various experiments (Choi et al. 1991; Dahl and Malo 2009a, Dahl and Malo 2009b; Franke et al. 2007; Garcia and Orteu 2001; Maas and Hampel 2006; Guindos and Ortiz 2013; Xavier et al. 2012). In contrast to the traditional techniques, the photogrammetric approach could provide more details of angular deformation, which are necessary for providing more details of shear deformation of torsion specimens. It provides a permanent visual and measurement record for further assessment. Optical methods that give displacements or strain fields are now widely used to evaluate material properties of timber-based composite. However, most of these methods have been extremely limited to give in-plane measurements, meaning that the results are not fully applicable to real structural systems.

This paper presents the experimental studies performed to investigate the feasibility of the developed photogrammetric system on the torsion test. Torsion tests were conducted on glulam timber beams according to the BS EN 408:2010+A1:2012 (2012) procedure of the standard torsion tests. The main purpose of this study is to provide an effective alternative method for evaluating the values and the variation of the shear modulus of glulam beams. Another objective of this study was to evaluate torsional behaviour

of glulam timber beams and to investigate the effect of shear spans on the values of shear modulus. This may assist in understanding the torsional behaviour of the specimens and give a basis for further studies.

## 2 Shear modulus from torsion test

The torsion test method has been included in the most recently updated British Standard, BS EN 408:2010+A1:2012 (2012) to obtain the shear modulus of timber. As described in the standard, the relative rotational displacements and the torque must be measured at two sections, which are spaced within the free testing length. Based on the Saint–Venant torsion theory of rectangular cross-section, the value of the shear modulus for each torsion specimen ( $G_{Tor}$ ) can be calculated using the equation given in BS EN 408:2010+A1:2012 (2012):

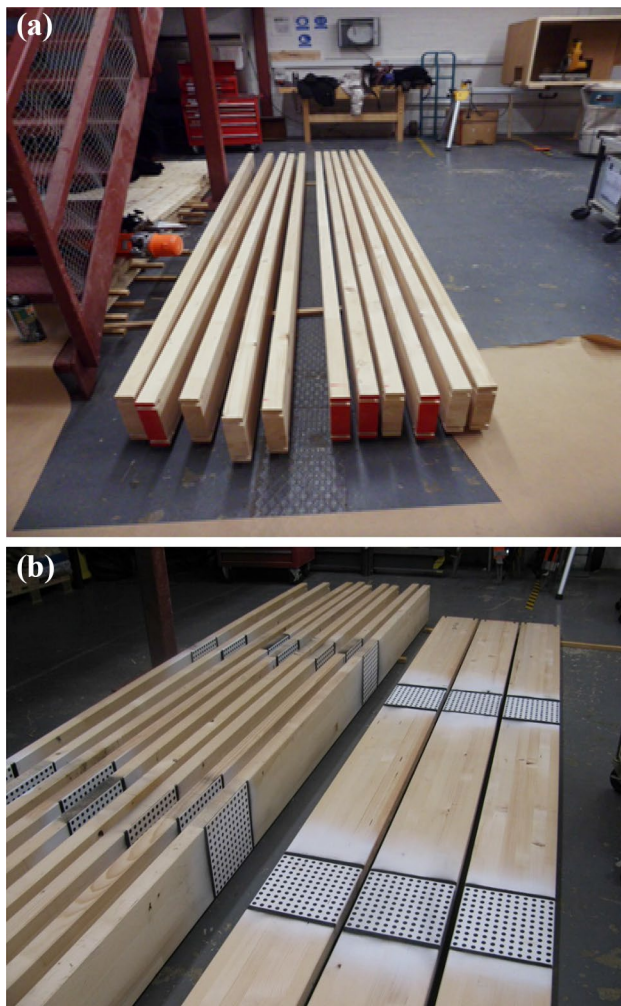
$$G_{Tor} = \frac{K_{Tor} l_1}{\eta h b^3} \quad (1)$$

where  $l_1$  is the gauge length and  $K_{Tor}$  is the torque stiffness, which can be determined using a linear regression analysis conducted on the linear elastic portion of the graph of applied torque within proportional limits and relative twist of the specimen.  $h$  and  $b$  are the depth and width of the specimen, respectively.  $\eta$  is a shape factor, which depends on the depth to width ratio and can be determined from a Table given in BS EN 408:2010+A1:2012 (2012).

## 3 Materials and methods

The rectangular cross-section timber glulam beams used in this study as samples for the torsion tests had cross sectional dimensions of about 45 mm × 220 mm and the total length of each beam was about 4 m, as shown in Fig. 1a. These beams are graded to GL24 class strength. This grade of glulam beams was selected because it is common and regularly available in the UK. As described in BS EN 408:2010+A1:2012 (2012) standard, the glulam samples were stored in a conditioning room with a constant temperature of 20 °C and a relative humidity of 65%.

Before testing and in order to employ the stereo camera systems for actual angular measurements, the glulam timber beams were prepared by applying photogrammetric targets to their surfaces. This was achieved by painting the area of interest with a white paint to serve as a background, which can assist in distinguishing the targets during image processing. Then, black circular marks (targets) were painted on the background. These targets are used to measure the rotations along the depth of the test specimen. Circular marks



**Fig. 1.** Glulam beam samples before and after photogrammetric patterns painted

are preferred because their centre point can be determined with high accuracy (Steger et al. 2008). The photogrammetric patterns consist of 121 circular targets arranged in 11 columns and 11 rows. These targets were marked 20 mm centre to centre to give traceable marks on the sample surfaces. The radius of the circular targets is 10 mm and the central target was set to be an origin of the X–Y coordinate system. Figure 1a shows an image of these glulam beams. In order to calculate the cross-sectional rotation, a straight cross-sectional reference line was linearly fitted from the 11 coordinates measured for each column. All the glulam samples after being prepared with the photogrammetric targets are shown in Fig. 1b.

A Tinius-Olsen (Tinius Olsen, Pennsylvania USA) torsion testing machine capable of applying up to 1 kilo newton-metre (kN-m) torque was used to perform the torsion tests on the specimens. Inclinometers (Model IS-2-30, Level Developments, UK) were employed to measure the angles

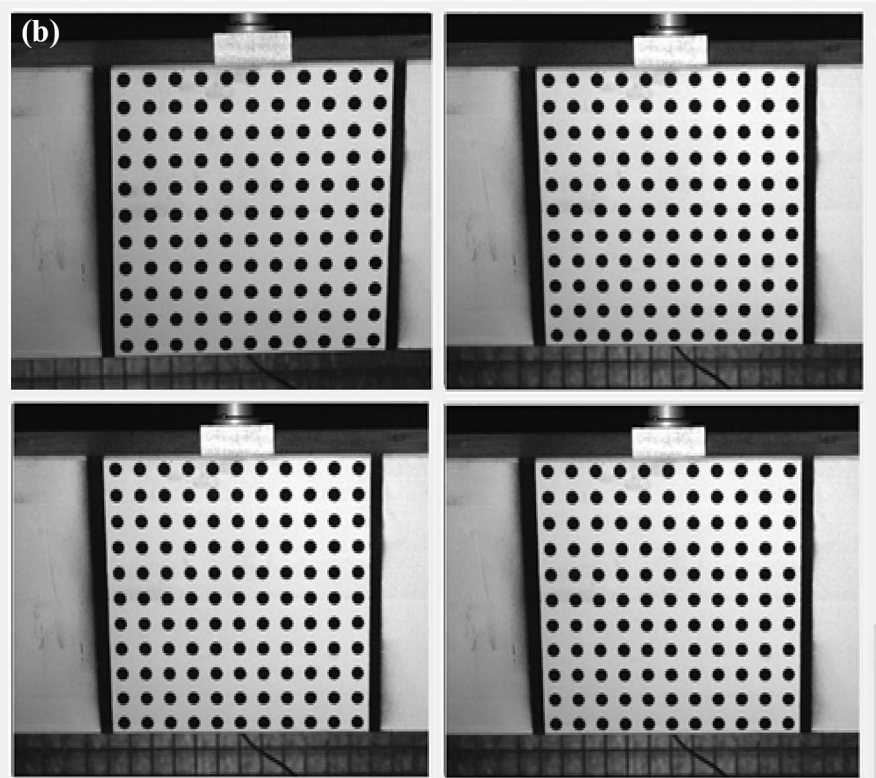
of twist of the glulam beams at the attached points. The IS-2-30 is a dual axis sensor capable of measuring angles with a measurement range of  $\pm 30^\circ$  in two directions with an accuracy of  $\pm 0.05^\circ$ . The diameter of the circular base is 43 mm. These inclinometers can measure the relative twist angle in both X and Y directions. A data log interface, as shown in Fig. 2, was developed by the authors of this study to simultaneously display, record and transfer data received from the inclinometers to a desktop computer, depending on the time interval specified by the user of the system.

In addition to inclinometers, two binocular stereo camera systems, as shown in Fig. 3a, were used to measure the actual rotational deformations of the long side of the cross section of the glulam specimens. Each of these sensors consists of two DMK 23GP031 cameras with a total resolution of  $2592 \times 1944$  pixels and high-speed CMOS area scan sensor. These cameras were fitted with an Azure-3514M5M lens having a constant focal length of 35 mm and pixel size of  $2.2 \mu\text{m}$ . Additionally, two LED light sources were employed to assist in providing a homogenous illumination of the stereo images. To capture the rotational deformations of the targets in real time, the authors of this research implemented MFC-based application, which was developed based on visual C++ 2010, providing the functions necessary to calibrate the binocular system and to record the image data during testing. The graphical user interface (GUI) of the developed application, as shown in Fig. 3b, makes use of libraries included in the image processing software, HALCON (MVTech Software HALCON GmbH 2019). These include functions not only for calibrating the binocular stereo system easily and accurately but also for 3-dimensional image photogrammetry.

## 4 Experimental setup and procedure

Figure 4 shows a schematic of the components of the torsion test setup and an image of this experimental setup is shown in Fig. 5. The glulam specimen was inserted in the torsion machine and four inclinometers were attached to the upper surface of the torsion sample. For validation purposes, inclinometers B and C were placed exactly above the photogrammetric patterns. The stereo cameras, Basler Pilot piA2400-17gm camera, 5 M pixel, Monochrome 2/3" CCD with GigE fast Ethernet interface, were mounted on a tripod and the baseline distance was set at about 25 cm. The width and height of a cell on the CCD-chip are all  $2.2 \times 10^{-3}$  mm. The two stereo camera systems were placed in front of the left and right photogrammetric pattern, respectively at a distance around 2.5 m away from the beam. The LED light sources were focused on the region of interest to negate the influence of natural and indoor lighting and to provide a homogenous illumination of the torsion sample. Prior to all

**Fig. 2** The purpose-built dual-axis inclinometer data log system



**Fig. 3** **a** Binocular stereo camera systems, **b** screen shot of the target areas from four cameras.

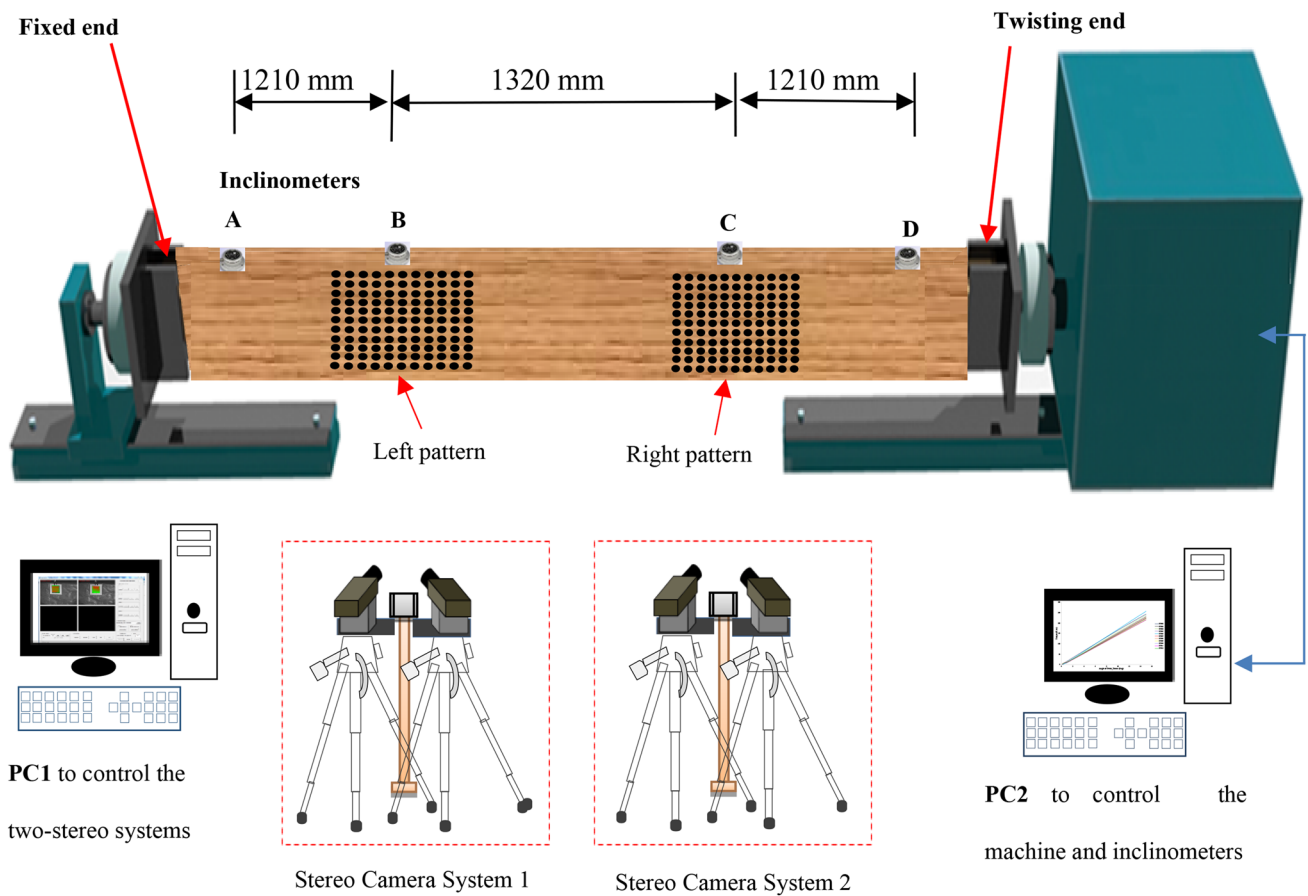


Fig. 4 Torsion test and dual stereo vision cameras setup



Fig. 5 Laboratory photograph of torsion test and dual stereo vision cameras setup

torsional tests, the two camera systems were calibrated using a 100 mm × 100 mm calibration plate and based on the stereo camera calibration of HALCON. The standard calibration plate was chosen according to the estimated dimensions of the region of interest to be captured as described in MVTech Software HALCON GmbH (2019). This target consists of

49 marks whose 3D coordinates are known with a high accuracy. The coordinates of these targets are given in the calibration description file which contains the geometrical information of the calibration plate but different for each size of the calibration plate as mentioned in the technical reference of MVTech Software HALCON GmbH (2019). More information about the calibration of the stereo cameras is given in detail in Steger et al. (2008). Once the binocular stereo systems were calibrated, the torsion tests were carried out four times for each torsion specimen according to the BS EN 408:2010+A1:2012 (2012) procedure of the standard torsion tests.

Preliminary torsion tests were carried out on the first sample to determine the rate of loading and to estimate the elastic range of the sample in order to ensure that no permanent deformation occurred during testing under torsional loading. Each sample was tested in the same manner; by applying a torque to the specimen, in the clockwise direction within the elastic range, at a constant rate of twist equal to 5°/min both in loading and unloading until the torsion angle of the tester reached 15°. The direction of the torsion torque was then reversed to an anticlockwise direction with the same loading rate until the torsion angle of the tester reached 15°. Finally, the loading

**Table 1** Internal camera parameters of the first stereo camera system

Camera Parameter	Description	Unit	Left Camera	Right Camera
Focus	Focal length of the lens	m	0.035	0.035
Kappa	Radial distortion coefficient	m <sup>-2</sup>	31.57	- 14.16
Sx	Width of a cell on the CCD-chip	m	2.2e-006	2.2e-006
Sy	Height of a cell on the CCD-chip	m	2.2e-006	2.2e-006
Cx	X-coordinate of the image centre	Pixels	1503.89	1424.04
Cy	Y-coordinate of the image centre	Pixels	957.12	992.70
Image width	Width of the images	Pixels	2592	2592
Image height	Height of the images	Pixels	1944	1944

**Table 2** 3D pose parameters (rotation and translation)

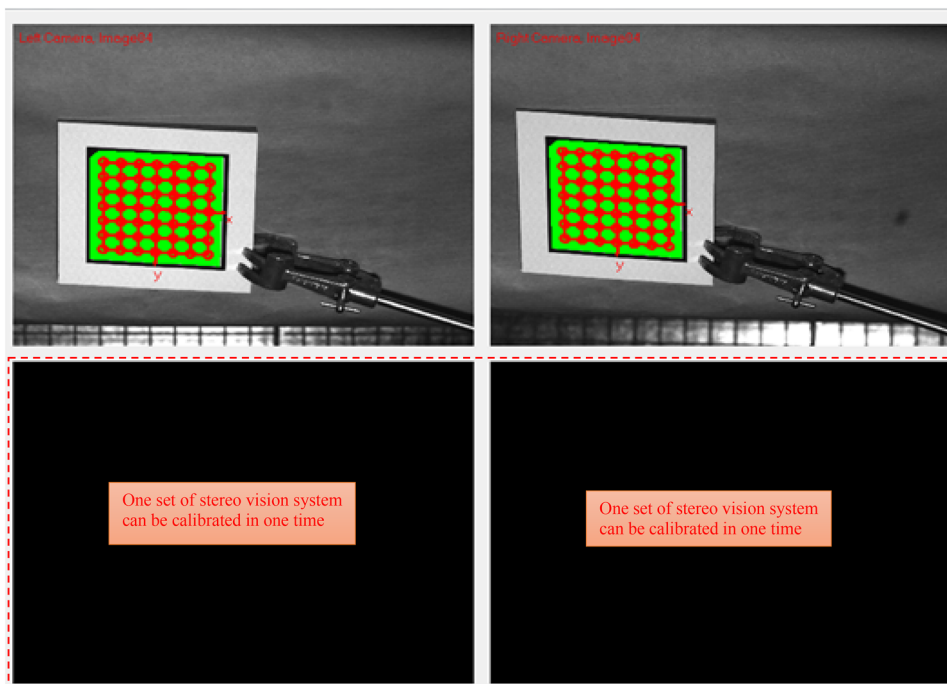
Translation vector (X Y Z [m])	X	0.175
	Y	- 0.008
	Z	0.000
Rotation angles (°)	Rot(X)	359.68
	Rot(Y)	355.53
	Rot(Z)	359.16

chuck was then returned to its initial test position to allow the test specimen to be removed from the machine. During the torque application, the implemented software was automated to record stereo images of the two regions of interest of the sample at a time interval of 5 s. The implemented GUI developed for the inclinometers was also employed to record their reading during the period of the test.

## 5 Experimental results and discussion

### 5.1 Calibration results of the stereo camera systems

In order to reconstruct the 3D coordinates of the photogrammetric targets captured by the stereo cameras, and hence determine their rotational deformations, the calibration parameters of the stereo camera systems were determined by performing the binocular stereo camera calibration of each system using the image processing software. This was achieved via a least square solution with suitable chosen parameters of the cameras that can be determined from the specification of the camera sensor and lens. Tables 1 and 2 show selected examples of the internal and external parameters, including the rotation angles and the translation vector determined from the first stereo system. A screenshot of the developed GUI during the process of calibrating the stereo camera system is shown in Fig. 6.

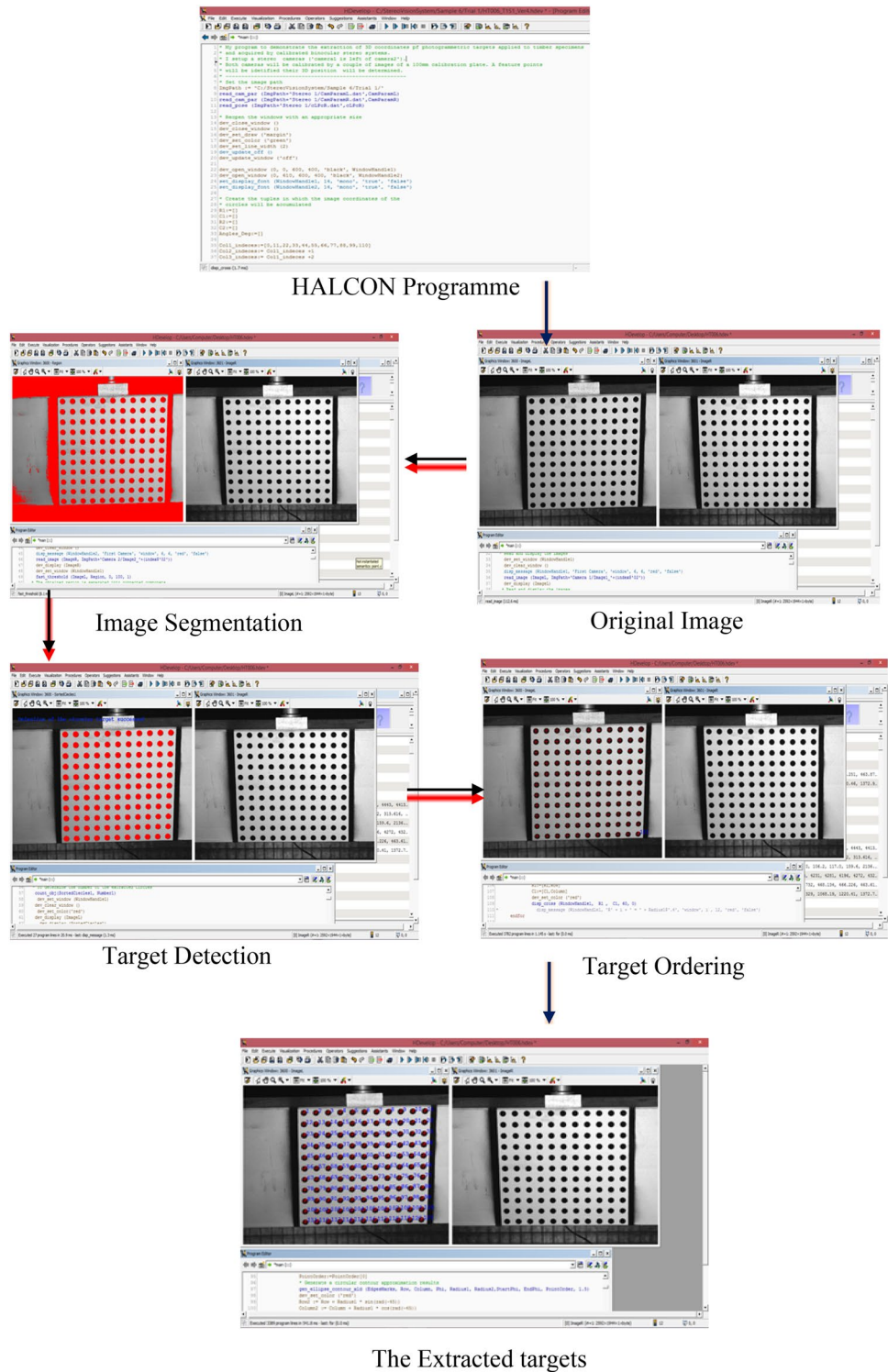
**Fig. 6** Calibration of the stereo vision system.

### 5.2 Target tracking and 3D reconstruction

The image processing algorithm is based on the comparison between two images acquired during the test, one

before and after deformation. Figure 7 demonstrates the concept and steps involved in the algorithm. The process starts with reading the calibration files and displaying the stereo images of the targets recorded from the left and

Fig. 7 Indicative flowchart for photogrammetric measurement

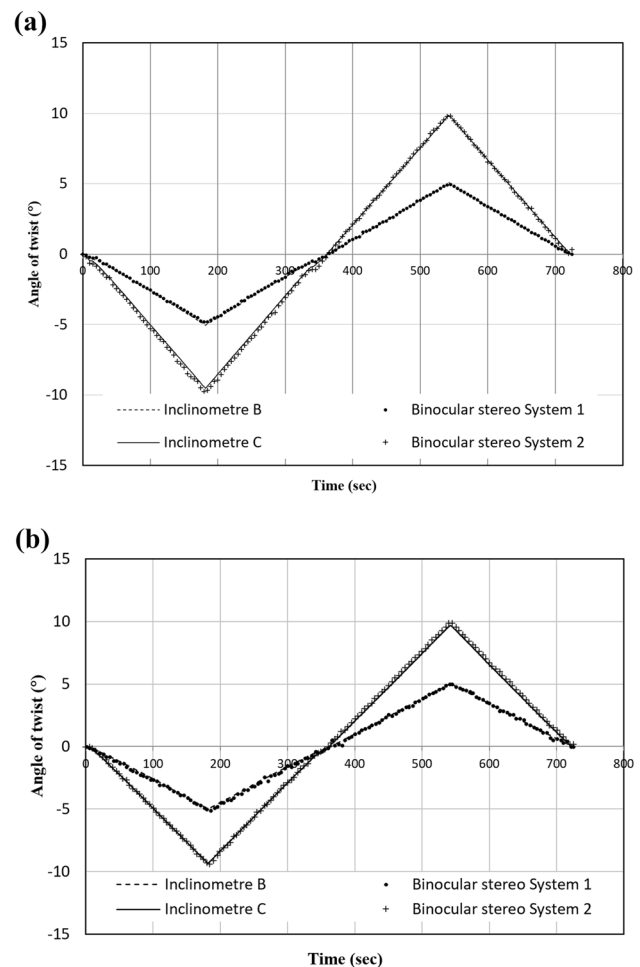


right cameras during testing. Based on their shape and size, the potential targets were determined and tracked from the two sequences of stereo images using the sub pixel-precise segmentation method. These targets were fitted to ellipse shapes and their image coordinates in both the left and right images were measured. The 3D coordinates of the targets expressed in the camera coordinates of the left camera were determined using the theory of triangulation (Hartley and Zisserman 2003). These coordinates were then used in the calculation of the displacements and rotations of the targets during the period of tests. This was achieved by finding the pixel coordinates of the targets of each pattern and averaging these values to define a point with the average pixel coordinates of the pattern. The pixel coordinates of the other two points were determined for each pair of the stereo images by processing the initial images before applying torque to the specimens. Utilizing the calibration parameters of the stereo camera system and triangulation, the 3D coordinates of the three points were determined to define two vectors. Using the two vectors, the rotations of the photogrammetric patterns for each time interval were then calculated.

### 5.3 Comparison of angular deformations measured by inclinometers and the photogrammetric systems

To understand the differences between the rotations of the long and short sides of the cross section, the rotations of photogrammetric patterns 1 and 2 and the rotations of inclinometers B and C were plotted in the same figure. These data were measured in both clockwise and anti-clockwise directions under torque loading and unloading phases. Figure 8a and b show some selected examples of these comparisons and indicate remarkable results. It is evident that a good agreement between the two methods has been observed and this indicates that the measurements of the rotations of the short side with inclinometers are acceptable.

To investigate the correlation between the measurements, the rotation results of the inclinometers and stereo camera systems were plotted against each other and a very strong correlation was observed for each glulam timber specimen. Selected examples of this comparison are shown in Fig. 9a and b in which the rotations of specimen 4, measured in the torsional direction by inclinometers B and C and by the first and second binocular stereo systems, were plotted against each other. These figures provide further evidence of this correlation when considering the correlation of determination ( $R^2$ ) of the angular measurements of inclinometers versus stereo camera systems. From the results shown in these figures, a high degree of correlation, with an R-squared value of 0.999, was observed.

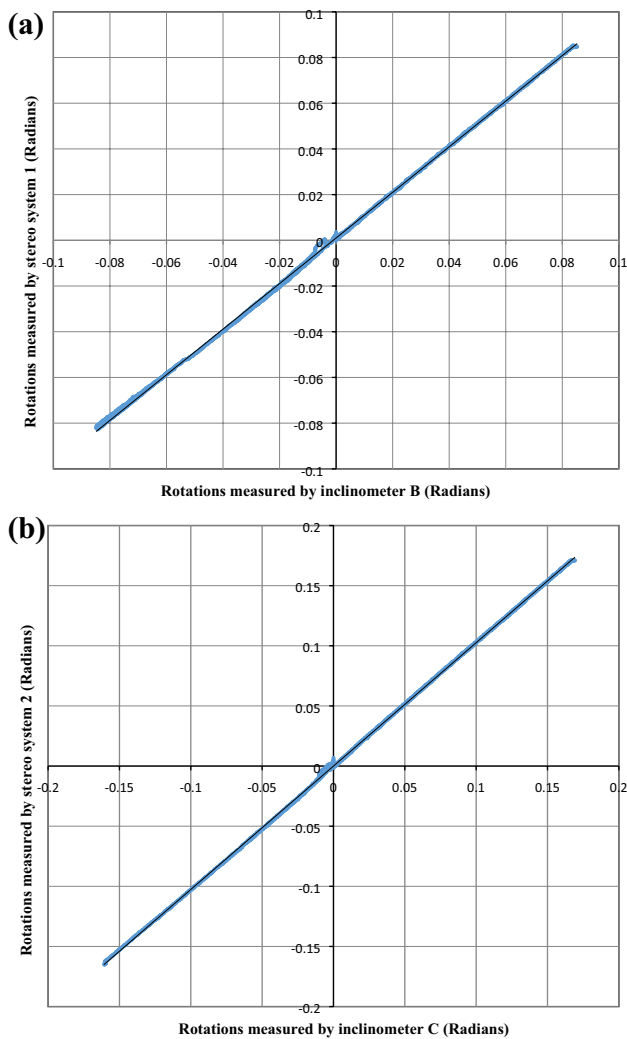


**Fig. 8** Comparison between inclinometers and optical measurements for **a** sample 5, and **b** sample 9 in the first torsion test

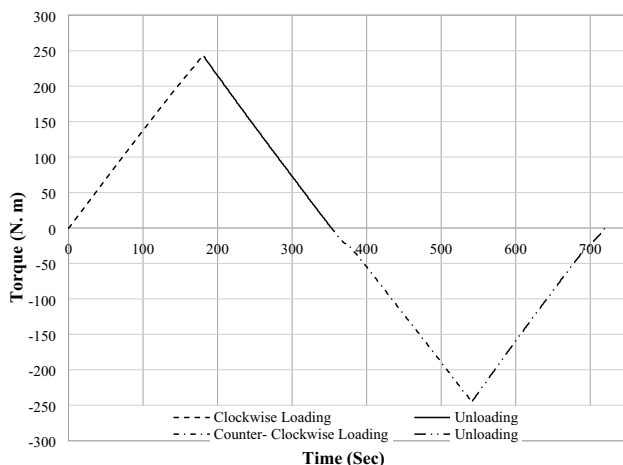
### 5.4 Torsional behaviour of the glulam timber beams

The results of preliminary tests that were carried out on specimen 1 revealed that the maximum torque applied to the specimen was within the linear-elastic range, about 250 N m, and the torsion specimen could be twisted by up to 15° before it began to behave non-linearly. This result was found from the inclinometers, which were mounted near the reactional supports. Based on this finding, torsion tests were conducted four times on each of the remaining eleven specimens (2–12) under cyclic loading in the clockwise and anticlockwise directions. The recorded torques measured from the torque cell of the torsion machine, were analysed and displayed graphically. As an example, the torque versus time graph obtained from the first torsion test conducted on glulam specimen 2 under loading and unloading phases is shown in Fig. 10. This plot indicates that the relationship between the values of torque and the corresponding time is





**Fig. 9** Comparison of the rotations measured in specimen 4 between **a** inclinometer B and the stereo camera system 1, and **b** inclinometer C and the stereo camera system



**Fig. 10** Responses of sample 2 to torques during the four phases of loading in the first torsion test

approximately linear during the different phases of loadings, confirming that the torsion specimen was within the elastic range. All the tested specimens deformed within the elastic range and exhibit the same linear behaviour, although there is a large degree of variability in the maximum values of torque applied to twist the torsion specimens.

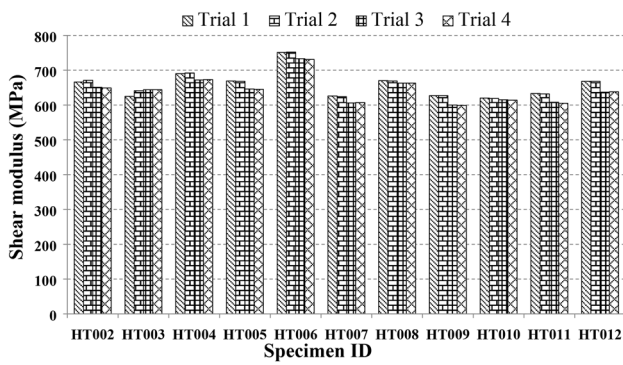
### 5.5 Torsional shear modulus of the beams based on the data of inclinometers

Using Eq. (1), the shear modulus of the glulam specimens was calculated for each loading–unloading phase in both clockwise and anticlockwise directions and the average value of  $G$  was determined by averaging the four measured values of shear modulus from all phases. Table 3 summarizes the values of the shear modulus of the glulam specimens calculated from each loading–unloading phase in both clockwise and anticlockwise directions. The average values of the shear modulus of each timber specimen, which was determined by averaging the four measured values of shear modulus from all loading and unloading phases, are also listed in the table. As specified in Table 3, one can easily notice that for each trial of test for the same sample, the measured value of shear modulus is different. It is unclear the cause of the phenomenon. However, it does suggest that one single torsion test specified in BS EN 408:2010+A1:2012 (2012), may not be the best way of evaluating the shear modulus of the timber beams. The calculated shear modulus ranges from 593 to 757 MPa with a mean value of 649 MPa. The difference can be explained by the fact that inhomogeneity and the variability of the wood specimens, as well as the presence of defects such as knots, affect the overall stiffness of the wood specimens. The average values of shear modulus for each torsion test are very close to each other, although there is a small difference between the values measured in the first two tests and those measured in the last two tests. These results are provided in Fig. 11, which graphically represents the average shear modulus of the test specimens calculated in each trial test. For all torsion specimens, the average values of the shear modulus calculated in the first and second trials are approximately equal, and this is true for those measured from the third and fourth trials. All these values are in good agreement and relatively close to each other with a difference range of 1–5%. The shear modulus’ values obtained from the first two torsion tests are relatively higher than those obtained from the last two torsion tests.

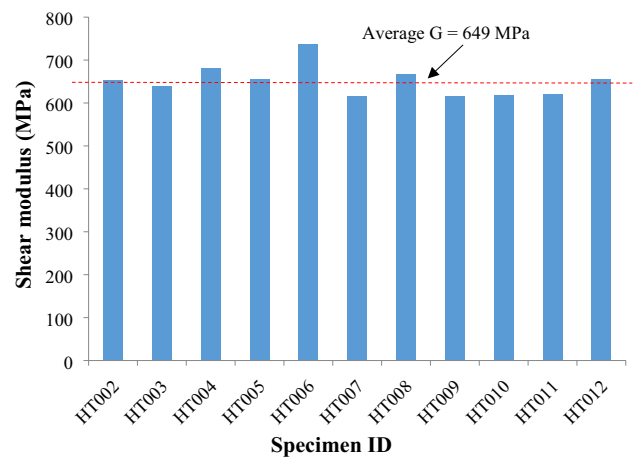
Table 4 presents the experimental average values of the shear modulus of each specimen calculated from all torsion tests for loading and unloading in both the clockwise and anticlockwise directions. The  $G_{Avg}$  indicated in this table represents the average shear modulus of all loading cycles of a test specimen, whereas  $G_{Avg-dir}$  represents the average shear modulus of each loading cycle of all tested specimens.

**Table 3** Shear modulus of the torsion samples in clockwise and anti-clockwise directions

Average shear modulus (MPa)							
Specimen ID	Trial no	Clockwise direction		Anti-clockwise direction		Average	COV%
		Loading	Unloading	Loading	Unloading		
HT002	1	670	671	667	654	666	1.20
	2	669	671	670	674	671	0.32
	3	644	653	655	652	651	0.76
	4	642	653	648	651	649	0.74
HT003	1	627	625	623	627	625	0.29
	2	643	641	639	640	641	0.23
	3	643	647	641	647	644	0.46
	4	642	648	641	646	644	0.52
HT004	1	700	699	680	681	690	1.54
	2	697	702	687	684	692	1.26
	3	675	673	670	670	672	0.36
	4	672	673	675	670	673	0.30
HT005	1	672	673	662	669	669	0.72
	2	669	673	662	668	668	0.65
	3	644	650	647	644	646	0.44
	4	641	649	647	645	645	0.52
HT006	1	753	755	751	744	751	0.62
	2	750	756	757	744	752	0.76
	3	729	737	735	731	733	0.49
	4	726	737	728	732	731	0.64
HT007	1	627	626	627	623	626	0.28
	2	623	625	627	622	624	0.37
	3	606	610	605	604	606	0.45
	4	605	610	606	605	607	0.40
HT008	1	671	674	666	669	670	0.51
	2	670	672	664	668	669	0.50
	3	668	671	657	654	663	1.21
	4	666	671	658	657	663	1.06
HT009	1	626	626	622	633	627	0.72
	2	627	627	624	632	627	0.54
	3	598	605	595	602	600	0.73
	4	598	606	593	602	599	0.94
HT010	1	628	630	610	613	620	1.64
	2	626	629	611	612	619	1.52
	3	613	617	614	613	615	0.30
	4	613	618	614	613	614	0.37
HT011	1	638	638	626	627	633	1.06
	2	636	641	627	625	632	1.16
	3	608	613	602	611	608	0.78
	4	604	609	601	608	605	0.66
HT012	1	670	673	662	667	668	0.71
	2	669	671	664	667	668	0.45
	3	638	638	633	639	637	0.40
	4	635	641	636	639	638	0.38



**Fig. 11** Average shear modulus of the torsion specimens calculated from each torsion test



**Fig. 12** Average shear modulus of the torsion specimens

From the loading and unloading cycles, the average values of shear modulus range from 648 to 655 MPa with a consistent standard deviation of (37–39) and a coefficient of variation (COV) of (5.7–6.1%). The small and consistent values of COV may indicate that the torsion test is suitable for determining the shear modulus since the torsion specimen will be under pure torsion.

Figure 12 demonstrates a graphical representation of the average shear modulus obtained for all glulam beams. In this figure, the average G results for each of the 11 glulam beams (2–12) listed in Table 4 are compared relative to the average shear modulus of all beams. The overall average G of all the fabricated glulam beams tested in torsion is 651 MPa. This value agrees well with the published value of the same board of glulam beam GL24 and conforms to the requirements of EN 14080 (2013).

### 5.6 Effects of the direction of the applied torque and loading phases on the values of shear modulus

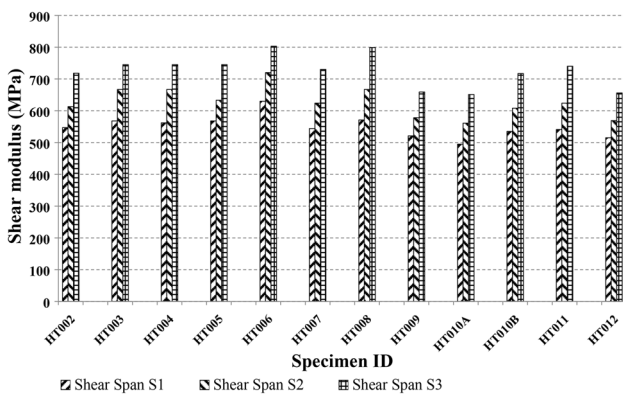
To assess possible misalignment of the test specimens and to observe whether inducing torque in a clockwise or anticlockwise direction influences the torsional behaviour and hence the shear modulus of the specimens, the torsion tests were conducted on each torsion specimen under cyclic loading. Each torsion test consists of phases of torque loading and unloading in a clockwise direction, followed by loading and unloading phases in an anticlockwise direction. From the results of shear modulus presented in Table 5, it can be observed that the experimental values of shear modulus

**Table 4** Average shear modulus of each of the torsion samples in all trial tests

Specimen	Average shear modulus (MPa)						Difference $G_{\text{clock-Ganticlock}}$	$G_{\text{Avg}}$
	Clockwise direction			Anticlockwise direction				
	Loading	Unloading	$G_{\text{Avg Clock-dir}}$	Loading	Unloading	$G_{\text{Avg Anticlock-dir}}$		
HT002	656	661	659	659	659	659	- 1	659
HT003	639	640	640	636	640	638	2	639
HT004	686	687	687	678	676	677	10	682
HT005	656	661	659	655	656	655.5	3	657
HT006	740	746	743	743	738	740.5	3	742
HT007	615	618	617	616	614	615	2	616
HT008	669	672	671	661	662	661.5	9	666
HT009	612	616	614	608	617	612.5	2	613
HT010	619	623	621	612	613	612.5	9	617
HT011	621	625	623	614	618	616	7	620
HT012	653	656	655	649	653	651	4	653
$G_{\text{Avg-dir}}$	651	655	653	648	650	649		651
Std	38	38	38	39	37	38		
COV	0.058	0.059	0.058	0.061	0.057	0.059		

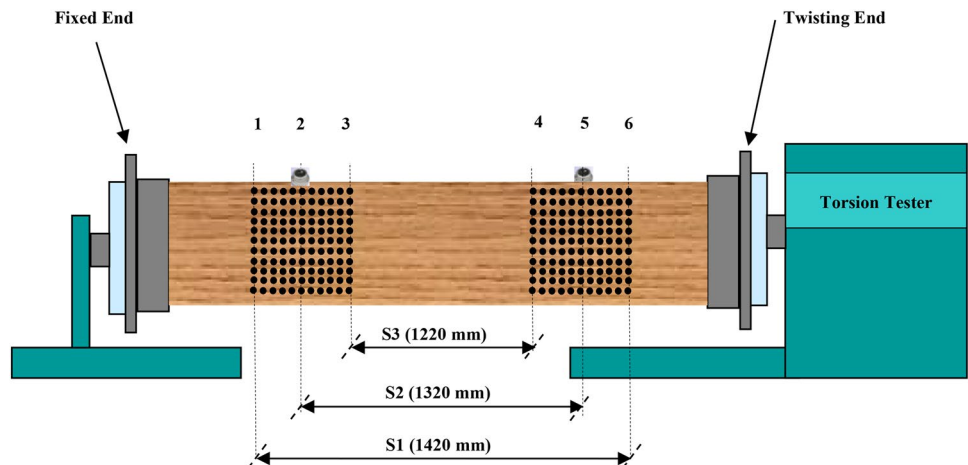
**Table 5** Shear modulus values of three spans of torsion specimens

Specimen ID	Shear span			$G_{Avg}$
	S1	S2	S3	
HT002	547	613	718	<b>626</b>
HT003	568	667	745	<b>660</b>
HT004	562	667	745	<b>658</b>
HT005	568	633	745	<b>649</b>
HT006	630	720	803	<b>718</b>
HT007	544	624	730	<b>633</b>
HT008	571	667	799	<b>679</b>
HT009	521	578	659	<b>586</b>
HT010A	495	561	651	<b>569</b>
HT010B	535	608	717	<b>620</b>
HT011	541	624	740	<b>635</b>
HT012	515	569	656	<b>580</b>
$G_{avg-span}$	<b>550</b>	<b>628</b>	<b>726</b>	<b>634</b>



**Fig. 13** Variations of shear modulus at different shear spans

**Fig. 14** Diagram of the different shear spans for determining the shear modulus on various segments



measured for all trial tests conducted on each torsion sample are consistent regardless of the direction of the torque or the loading phases. However, there is a small deviation, which is expected for engineered wood products such as glulam beams. As mentioned before, the experimental values of shear modulus are in good agreement and are relatively close to each other with a difference range of 1–5%. This may indicate that shear modulus is less sensitive to torque direction as there was no significant influence of the torque direction on the measured shear modulus.

### 5.7 Effect of shear span on the variation of shear modulus

The torsion specimens were non-destructively evaluated under torsional loading applied within the elastic range. As a result, it becomes essential to examine the effect of span variations on the shear modulus at different cross-sections. Therefore, based on the shape of the photogrammetric patterns, shown in Fig. 13, three shear spans S1, S2, and S3 at the middle section of the torsion specimens were considered. The first shear span, S1, is the distance between Sects. 1 and 6, representing a gauge length of 1420 mm. The second shear span, S2, is the distance between Sects. 2 and 5 (the distance between inclinometers B and C), representing a gauge length of 1320 mm and the third shear span, S3, is the distance between Sects. 3 and 4, representing a gauge length of 1220 mm.

In addition to the stereo images of the photogrammetric patterns, the values of torque were recorded continuously during testing. The relative rotation of each of the two cross-sections over their gauge length (shear span) was measured using the 3D coordinates of the targets distributed on the longer side of the cross section. Using these data, in addition to the geometry of the torsion specimens, the shear modulus values for each shear span were calculated using Eq. (1).

**Table 6** Average shear modulus of specimen 10 calculated in different ways

Specimen ID	Test no	Average shear modulus (MPa)				Average	COV %
		Clockwise direction		Anti-clockwise direction			
		Loading	Unloading	Loading	Unloading		
HT010A	1	628	630	610	613	620	1.64
	2	626	629	611	612	619	1.52
	3	613	617	614	613	615	0.30
	4	613	618	614	613	614	0.37
HT010B	1	608	612	609	618	612	0.71
	2	607	613	608	619	612	0.86
	3	629	634	624	629	629	0.69
	4	628	633	623	629	628	0.68

Four torsion tests were conducted on each torsion specimen and the results were averaged. From these results, values of  $G$  were obtained for the three shear spans. Table 4 presents the calculated average values of the shear modulus of the three regions of each specimen. The  $G_{Avg}$  indicated in this table represents the average shear modulus of all shear spans of a test specimen, whereas  $G_{Avg-span}$  represents the average shear modulus of each shear span of all tested specimens. From Table 4, it can be observed that the average shear modulus varies significantly when considering the three different shear spans of each timber beam.

The largest shear modulus appears to occur at the lowest shear span, which is far away from the gripping. On the other hand, the largest span, which is very close to the gripping, attained the smallest shear modulus. The same trend of variation was observed in all test specimens. This result can be clearly observed from the charts plotted in Fig. 14. The values of the shear modulus calculated in the shear span S2 are 10–16% higher than those calculated in the larger span S1 and about 12–17% lower than those calculated in the smaller span S3. For instance, the shear modulus of 613 MPa was attained for shear span S2 of specimen 2, which was 11% higher than the average shear modulus of S1 and 15% lower than the average shear modulus of S3. The main reason for this effect could be that the larger span considered in this study is between two sections and one of them is close to the rotating end of the test specimen. The rotation angles affect the shear stiffness and hence the values of shear modulus. The angle of twist in each length is inversely proportional to the shear modulus. From these results, and within the selected shear spans, it can be concluded that the larger the shear span the smaller the value of shear modulus.

## 5.8 Influence of inverting the glulam samples

To investigate the effects of inverting the torsion samples on the torsional behaviour, sample 10 was tested under torsional loads in two ways: the right way up and upside down. Therefore, two names, 10A and 10B, were defined for this specimen to define the right way up and upside down, respectively. Table 6 reports the average shear modulus of this specimen calculated in both orientations. When installing torsion specimens, the right-side up, the average values of shear modulus calculated from the first and second trials are relatively higher than those measured from the third and fourth trials. On the other hand, when inverting specimen 10, the average values of the shear modulus calculated from the first and second trials are relatively smaller than those calculated from the third and fourth trials. It can be concluded that mounting the torsion specimens the right way up during the torsion test yields higher values of shear modulus than those obtained when inverting the specimen, although this finding was based on only a single piece of glulam beam.

## 5.9 Discussion on the torsion test method and photogrammetry method

Although the torsion test method may be a better approach to determine the shear properties of structural-size timber-composite materials, this method has rarely been employed for the determination of  $G$ . This is due to the lack of information available for proper use of the torsion test method, in addition to the difficulty of measuring reliable angular deformations of the test specimen using the torsion testing machines, such as the Tinius Olsen Machine. Therefore, the photogrammetric approach was proposed for this type of measurement, aiming to provide further understanding of the torsional behaviour of the glulam beams.

This research describes the proposed non-contact photogrammetric approach and investigates its ability to evaluate the shear modulus of glulam timber beams. The research focused mainly on the experimental study of torsion on structural-size glulam beams. Based on the experimental work of this research, the following conclusions can be drawn:

The proposed method provides a cost-effective and non-contact method for determining reliable deformations at multiple locations on the tested specimen, which may assist in evaluating the values and variations of the  $G$  of glulam timber beams. This technique overcomes the limitations of traditional contact devices, which only measure the angular deformation on a restricted number of points on the surface of the torsion specimens and can be destroyed during destructive tests.

The test results showed that applying torques to the torsion specimens during loading and unloading in either a clockwise or anticlockwise direction does not influence their stiffness and hence their shear modulus.

The average values of the shear modulus of the laminated timber specimens ranged from 593 to 757 MPa. The difference can be attributed to the inhomogeneity and the variability of the wood specimens as well as the presence of defects such as knots which affect the overall stiffness of the wood specimens. The overall average  $G$  of all the glulam beams tested in torsion is 649 MPa. This value agrees well with the published value of the same board of glulam beam GL24 and conforms to the requirements of EN 14080 (2013), which gives characteristic material properties for homogenous and combined glulam members.

To investigate the effects of inserting the torsion samples upside down on the torsional behaviour, sample 10 was tested under torsional loads in two ways: the right way up and upside down. It was observed that mounting the torsion specimens the right way up during the torsion test yields higher values of shear modulus than those obtained when inserting the specimens upside down, although this finding was only based on a single piece of glulam beam.

The photogrammetric approach enabled the investigation of the influence of the shear span on the shear modulus of the tested glulam beams under the torsion tests. It was apparent that the shear modulus varied significantly with the different shear spans (12–17%). The test results of all the tested specimens revealed that the larger the shear span the smaller the value of shear modulus.

## 6 Conclusion and recommendations

This research quantifies values and variations of the shear modulus of the glulam specimens under the torsion test according to the BS EN 408:2010+A1:2012 (2012) standard method with the help of the developed photogrammetric method. Future research efforts that could be investigated based upon the results of this research include the following:

1. The torsion test method is recommended by BS EN 408:2010+A1:2012 (2012) to evaluate the shear modulus of structural-size specimens. However, this standard does not adequately address the test procedure in greater detail, especially the appropriate way to measure the rotational deformation of the torsion specimens. This investigation proved that the value and variations of the shear modulus can be obtained from the torsion tests by using the photogrammetric technique. The inclusive findings provided by the optical system on the use of the torsion test could contribute to the experimental test procedure described in the above-mentioned standard.
2. Since the shear modulus is a fundamental mechanical property of wood, it would be of great interest to apply the photogrammetric approach in the shear field test and investigate the correlation between the shear modulus and modulus of elasticity for the glulam timber beams.
3. The torsion tests were carried out on structural-size glulam timber beams. Although clear wood may not be a better approach to determine the shear properties of the timber beams, it would be of significant interest to conduct the torsion tests on clear wood specimens and investigate the correlation between shear modulus values of the clear wood and full-size specimens.

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