

Aerodynamic performances and vehicle dynamic response of high-speed trains passing each other

Yinghui ZHAO, Jiye ZHANG^{*}, Tian LI, Weihua ZHANG

Traction Power State Key Laboratory, Southwest Jiaotong University, Chengdu 610031, China

Abstract: Based on the aerodynamics and vehicle dynamics, the aerodynamic performances and vehicle dynamic characteristics of two high-speed trains passing each other on the ground, embankment and bridge are studied. Firstly, a train aerodynamic model and a vehicle dynamic model are established. Through the simulation of the two models, the pressure waves, aerodynamic forces, and vehicle dynamic responses are obtained. Then, the pressure waves and aerodynamic forces on different foundations are compared. The results show that the variation trends of pressure wave and aerodynamic forces of trains passing each other on different foundations are almost similar. The peak-to-peak differences in pressure wave and aerodynamic force are below 4% and 3% in three cases in open air. Besides, the differences of security indexes, including coefficient of derailment, wheel unloading rate, the wheelset lateral force, and the wheel-rail vertical force, are below 2% among the three cases; the differences of comfort indexes, including the lateral acceleration and the vertical acceleration, are also below 2%. It is concluded that the dynamic performances of trains passing each other are influenced little by different foundations in open air.

Key words: trains passing each other; foundations; pressure wave; aerodynamics; vehicle dynamics © 2012 JMT. All rights reserved.

1. Introduction

igh-speed train can induce strong disturbance to H the surrounding air. When two high-speed trains are passing each other in open air, the disturbance will be aggravated, which would induce very large transient pressure fluctuations [1-3]. The pressure wave will lead to the deformation and lateral movement of car body, even the destruction of the window glass, and then influence the running security of the trains and the passenger comfort [4-7]. Tian et al. [8] studied the behaviors of trains passing each other at different speed levels and different line spacings. The research proved that the amplitudes of pressure wave of trains passing each other are proportional to the train speed, and that the relationship between pressure wave and line spacing is negatively exponential. Aiming at the computational efficiency, Ref. [9] proposed a method with only a half of the computational domain to the simulation problems of trains passing each other at the same speed. Because only a half of the computational domain is required, and the dynamic mesh technique is avoided, the computational efficiency is greatly improved. In addition, Khier

Received Jan. 5, 2012; revision accepted Feb. 28, 2012 *Corresponding author. Tel.: +86-28-86466040 E-mail: jyzhang@home.swjtu.edu.cn (J.Y. ZHANG) © 2012 JMT. All rights reserved doi: 10.3969/j.issn.2095-087X.2012.01.006 et al. [10] studied the flow structure around trains under side wind condition by simulation and obtained the aerodynamic performances of train running under side wind condition.

Two trains passing each other may happen on the ground, embankment, and bridge in open air. However, few researchers studied the effects of different foundations on the train aerodynamics and vehicle dynamics when trains passing each other. In this paper, we investigate the influences of different open foundations (ground, embankment and bridge) on the dynamic behavior of trains passing each other. The aerodynamic performances and vehicle dynamic responses of trains are simulated with the commercial software Fluent and Simpack. The pressure waves, aerodynamic forces and vehicle dynamic responses are analyzed on different foundations in open air.

2. Aerodynamic model building

2.1 Governing equation of fluid dynamics

When the two trains are passing each other at the speed of 380 km/h, the air flow around the trains is compressible, unsteady and turbulent in nature with high Reynolds number. On the basis of the standard $\kappa - \varepsilon$ two-equation, the equations of the flow around trains are written as [11]:

$$\frac{\mathrm{d}}{\mathrm{d}t} \int_{V} \rho \phi \mathrm{d}V + \int_{S} [\rho \phi (U - U_g) - \Gamma_{\phi} \mathrm{grad}\phi] \mathrm{d}S = \int_{V} S_{\phi} \mathrm{d}V, \qquad (1)$$

where ϕ is flow flux, t is time, ρ is density of fluid, S is the area of control-volume interface, V is volume, U is the velocity of fluid, U_g is the velocity of controlvolume interface, Γ_{ϕ} is the diffuse coefficient, and S_{ϕ}

is the source term.

Ideal gas state equation is described as follows [12]:

$$p = \rho RT, \tag{2}$$

where *p* is pressure, ρ is density, *R* is gas constant, and *T* is temperature.

Finite volume method is used to solve Eqs. (1) and (2), and the second-order upwind difference formula is selected as the discrete format. Based on Eqs. (1) and (2), the pressure wave and aerodynamic forces of high-speed train are obtained by the simulation of the train aerodynamic model.

2.2 Geometry models

In order to study the behaviors of trains passing each other on different foundations (ground, embankment, and bridge), a certain high-speed train is chosen as the research object in this paper. The geometry models include train model, ground model, embankment model and bridge model. This paper studies the models of three-car marshalling and eight-car marshalling. The three-car marshalling model [13] includes one head car, one middle car, and one tail car. While the eight-car marshalling model includes one head car, six middle cars and one tail car. The head car and tail car have the same car appearance in both the cases. Moreover, in order to control the number of grids, the train is simplified as a 3D geometry with smooth surfaces, without considering the detailed features, such as pantographs, bogies and door handles, as shown in Fig. 1.

In reality, the structures of foundation include rails, sleepers, and so on. The sizes of those detailed structures are smaller than those of the whole track bed or the train. If considering all these details, the amount of calculation will be very enormous. Thus, the whole foundations are simplified as a 3D geometry with smooth surfaces, ignoring sleepers, track bed, and the detailed features such as bridge piers (Fig. 2).



(a) Eight-car marshalling (b) Three-car marshalling

Fig. 1 Simplified train model



Fig. 2 Simplified model of foundations (unit: m)

3. Verification of computational models

In order to monitor the pressure wave of the train surface in the whole process of trains passing each other, some monitoring points are fixed on the train aerodynamic model (head car, middle car and tail car) in simulation, as shown in Fig. 3.



Fig. 3 Monitoring points on the window and door

The pressure waves of trains passing each other obtained from experimental [14] and simulation are shown in Fig. 4 under the same conditions (both are eight-car marshalling with the same speed and foundation environment). The peak-values and variation trend of the simulation results are almost consistent with the experimental results. Therefore, the calculation method of eight-car marshalling model is feasible.

The pressure wave and head-car side force F_x of three-car marshalling model and eight-car marshalling model are shown in Fig. 5. By comparing the pressure wave and aerodynamic force of two models, it can be

seen that the variation trends of two models are almost consistent, and the difference in peak values of two models is about 3%.

The pressure waves and aerodynamic forces of middle car and tail car have the similar patterns. As a result, using the three-car marshalling model to simulate the problem will be more economic.









(d) Head-car side force of eight-car marshalling

Fig. 5 Comparison of three-car marshalling and eight-car marshalling

4. Analysis of simulation results of train aerodynamic performances

In the simulation, six cases are simulated to study the aerodynamic performances of the trains passing each other on the ground, embankment, and bridge. The speed of train is 380 km/h. The height of embankment is 5.0 m and 7.0 m, and the height of bridge is 10.0 m, 12.0 m and 15.0 m. The track spacing is 5.0 m and the height of track bed is 0.35 m.

4.1. Pressure wave of train passing each other

The pressure waves of monitoring points on head car, middle car and tail car when trains passing each other on the ground, embankment and bridge at the speed of 380 km/h are shown in Fig. 6. We can see that the pressure waves of the corresponding monitoring points have a similar pattern. When the head car of one train is passing the monitoring points of the other train, the monitoring point pressure will increase and then drop suddenly; when the tail car is passing the monitoring point, the pressure wave has the opposite regular pattern.



(b) Window of the head car







Fig. 6 Pressure waves of different foundations on open line

Comparison of the peak-to-peak values of pressure waves for trains passing each other on the ground, embankment, and bridge are shown in Fig. 7. It is revealed that the peak-to-peak value on bridge is relatively smaller than on the embankment and ground. The differences of peak-to-peak values of all monitoring points are below 4%, most of which are about 2%.





4.2. Aerodynamic forces of trains passing each other

The computation of aerodynamic force and moment is based on the coordinate in Fig. 8, side force F_x , lift force F_y , yaw moment M_y , and roll moment M_z . The aerodynamic performance of train 1 is chosen as the research object in this paper. The F_x , F_y , M_y , and M_z of head car passing by at the speed of 380 km/h are shown in Fig. 9. We can see that F_x , F_y , M_y , and M_z on different foundations have the similar fluctuation pattern. The differences of peak-to-peak values of all aerodynamic forces are below 3%, most of which are about 2%.



Fig. 8 Computation coordinate



Fig. 9 Aerodynamic forces of the head car on different foundations

5. Vehicle dynamic model building

In the dynamic modeling of high-speed trains, the suspension system and the vibration damper system are simplified to spring-damper force elements [15]. In this paper, we consider that the wheel-rail contact, wheel-rail force, yaw damper, lateral stop and the connection of spring and damper in series or in parallel are all geometrically linear in the vehicle dynamic model. The vehicle dynamic model chooses LMA and T60 as the wheel profile of the train and the rail respectively. The distance between backs of the wheel flanges is Chinese standard 1 353 mm, and the vehicle dynamic model chooses the measured track spectrum of Beijing-Tianjin high-speed railway as the track irregularity.

The equations of the train system dynamics are written as [16]:

$$M\ddot{X} + C\dot{X} + KX = F_r + F_a, \qquad (3)$$

where M, C, and K are the mass, damp and stiffness matrixes, respectively; X, \dot{X} , and \ddot{X} are the generalized displacement, velocity and acceleration vectors of the systems, respectively; F_x is the generalized load vector of the rail excitation; and F_a is the generalized load vector of the aerodynamics forces load. Based on Eq. (3), the vehicle dynamic responses (the coefficient of derailment, wheel unloading rate, the wheelset lateral force the wheel-rail vertical force, lateral acceleration and vertical acceleration) are obtained by the simulation of the vehicle dynamic model.

The coefficient of derailment, wheel unloading rate, the wheelset lateral force and the wheel-rail vertical force are used in the evaluation of running security of high-speed trains in this paper. The indexes should below these limitations: the coefficient of derailment $(Q/P)_{\text{lim}}=0.80$, wheel unloading rate $(\Delta P/\overline{P})_{\text{lim}}=0.80$, wheelset lateral force $H_{\text{lim}}=10+P_0/3$, (P_0 is the axle load)



Fig. 10 Safety index

and the wheel-rail vertical force $P_{\text{lim}}=170$ kN. In addition, lateral acceleration and vertical acceleration are used as the evaluation of passenger comfort of high-speed trains in this paper. The indexes should below these limitations: lateral acceleration $(a_y)_{\text{lim}}=2.5 \text{ m/s}^2$, vertical acceleration $(a_z)_{\text{lim}}=2.5 \text{ m/s}^2$.

6. Analysis of simulation results of vehicle dynamic responses

The indexes of security (coefficient of derailment,

wheel unloading rate, the wheelset lateral force and the wheel-rail vertical force) of high-speed trains passing each other on the ground, embankment and bridge at the speed of 380 km/h are shown in Fig. 10. We can see that the differences of all the security indexes are below 2%.

The comfort indexes (the lateral acceleration and the vertical acceleration) of high-speed passing trains on the ground, embankment and bridge at the speed of 380 km/h are shown in Fig. 11. We can see that the differences in all the comfort indexes are below 2%.



Fig. 11 Comfort indexes

7. Conclusion

Based on the analysis of the results, the following conclusions can be drawn:

(1) When two trains are passing each other on the ground, embankment and bridge, the pressure waves of the corresponding monitoring points have the similar regular pattern at the same speed; the differences of the peak-to-peak values are below 4%.

(2) The aerodynamic forces of the passing trains have the similar regular pattern at the same speed on the ground, embankment, and bridge; the differences of the peak-to-peak values are below 3%.

(3) The differences in all the indexes of comfort and security of trains passing each other on the ground, embankment, and bridge are below 2%.

(4) The train aerodynamics and vehicle dynamics of trains passing each other are influenced little by different foundations in open air.

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