# TECHNICAL NOTE



# 3D Numerical simulations of elastomeric bearings for bridges

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Abstract Multilayer elastomeric isolation can be considered a well-known solution with many applications in the last 20 years in the infrastructure arena. In particular, isolation has been extensively applied for pier and abutment protection, in order to strengthen bridges against earthquakes. Elastomeric bearings can be subjected to large axial loads and lateral displacements during strong earthquakes, which induce potentially buckling effects. The recent Forcellini and Kelly (J Eng Mech 140(6):04014036, 2014) model allows to take into account large deformation response of a bearing when buckling occurs. This paper aims at verifying this theory using experimental results and numerical simulations. First of all, test results are taken from Nagarajaiah and Ferrell (J Struct Eng 125:946-954, 1999) and compared with the developed theory. Then, numerical simulations have been performed by applying OpenSees.

**Keywords** Elastomeric bearings · Bridges · Isolation · OpenSees · Buckling

# Introduction

Past earthquakes all over the world have shown that the piers are the most sensitive parts of a bridge and their damage can be responsible for the collapse of the bridge spans. Therefore, pier protection should be considered as one of the most important goals for bridge protection and

Davide Forcellini davide.forcellini@unirsm.sm strengthening against earthquakes. As shown by Makris and Zhang [19] and Morgan and Mahin [21], seismic isolation is conceivably one of the most promising alternatives especially in post-earthquake rehabilitation. Several studies by Vlassis and Spyrakos [28] and Tongaonkar and Jangid [26] show the benefic effects of the isolation technique on bridge safety and design costs. Other studies by Ucak and Tsopelas [27] and Forcellini [10], analyzed the importance of considering non-linear hysteretic behaviors for the isolation systems.

This paper aims at modelling multilayered elastomeric bearings, widely used in civil engineering as seismic isolators for bridges. Generally, the bearings are used in the case of compression and somewhere in presence of shear force. Therefore, it is necessary to have an accurate knowledge of the behavior of such devices under these kinds of load conditions. In this regard, buckling has been widely investigated with experimental tests, such as [1–7, 15, 16, 24, 31] and numerical simulations [22, 25, 29, 30, 32].

Recently Forcellini and Kelly [12] described the buckling phenomenon extending the original two-spring model of the bearing, developed by Kelly [13] and [17] to large deformations. The model is used to illustrate the influence of large deformations on the interaction between horizontal and vertical loads and assessing the post-buckling behavior of these bearings. In particular, the previous study was conducted with the aim to extend the original linear theory of multilayered elastomeric bearings by replacing the differential equations by the algebraic ones. The model has been recently performed in order to study some applications [10].

This paper aims firstly at comparing several test results with the developed theory. In particular, results (under compression and shear loads) from Nagarajaiah and Ferrell

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[22] were considered and compared with the theoretical approach. In this regard, the actual state of mechanical computations, allows investigating high non-linear behaviors with sophisticated numerical tools.

The paper aims at calibrating a 3D finite element (FE) model able to reproduce elastomeric bearings complex behavior. In particular, OpenSees (Open System for Earthquake Engineering simulations) by the Pacific Earthquake Engineering Research (PEER) center has been applied here in order to reproduce elastomeric bearings' three-dimensional behavior. The finite element (FE) model has been reproduced with a layered system able to represent the alternating steel and rubber layers and the bolted connections. OpenSees is able to simulate realistic lateral boundaries built up by assuming shear beam conditions in order to reproduce the shear effect between the two materials. During lab tests, non-linear behaviors have been registered, as shown by Nagarajaiah and Ferrell [22]. In order to take into account such behavior, calibrated finite element models can be used as powerful tools.

## **Theoretical model**

The two-spring model (shown in Fig. 1) consists of two rigid elements in the shape of tees, connected by moment springs across hinges at the top and bottom and by shear springs and frictionless rollers at midheight [12]. The deformation variables shown are the shear displacement (*s*), the relative rotation ( $\theta$ ) and the horizontal displacement (*v*). The vertical load is *P* while the horizontal load at the top of the column is *F*. Following the formulation by Forcellini and Kelly [12], the kinematic at the top of the column in terms of horizontal displacement is defined by Eq. (1), where *h* is the height of the model. Equilibrium

Fig. 1 Two-spring model [12]

conditions allow calculating  $\theta$  from Eq. (2). The horizontal displacement can be evaluated by applying Eq. (3), fixing  $\lambda$  and p and solving for f as a function of the calculated  $\theta$ .

$$v = h \cdot \sin \theta + s \cdot \cos \theta \tag{1}$$

$$(p \cdot \cos \theta + \lambda) \cdot (p \cdot \sin \theta + f \cdot \cos \theta) = 0$$
(2)

$$\frac{v}{h} = \sin\theta + \frac{1}{\lambda}(p \cdot \sin\theta + f \cdot \cos\theta) \tag{3}$$

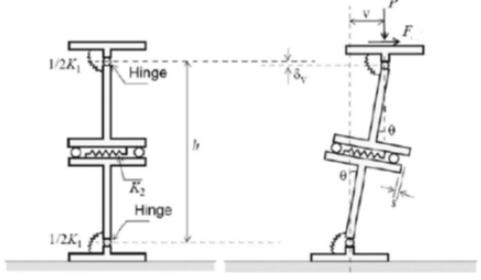
where:

$$P_E = \frac{\pi^2 \cdot E \cdot I_S}{h^2} \quad (EI_S \text{ bending stiffness}) \tag{4}$$

$$P_S = G \cdot A_s(GA_S \text{ effective shear stiffness})$$
(5)

$$p = \frac{P}{\sqrt{P_E P_S}}; f = \frac{F}{\sqrt{P_E P_S}}; \lambda^2$$
$$= \frac{P_S}{P_E} \text{ (usually between 0.001 and 0.05)} \tag{6}$$

In this paper, the multilayer elastomeric bearing has been taken from the ones tested by Nagarajaiah and Ferrell [22] and named 302 (square: width B = 5 in (127 mm), h = 4.385 in (111.4 mm), shape factor S = 5, 8 rubber layers (thickness: 0.25 in, 6.35 mm total) and 7 steel shims (thickness: 0.055 in, 1.397 mm total), as shown in Fig. 2. The bearing has a rubber cover of 0.125 in. (3.18 mm) and 1 in. (25.4 mm) thick end plates. The axial load was varied: 20 kip (89 kN), 30 kip (133 kN) and 40 kip (188 kN), 50 kip (222 kN). The mechanical characteristics (shear modulus G and bulk modulus K) of the rubber have been taken as 1.4 and 2000 MPa, respectively, in according with the values considered by Nagarajaiah and Ferrell [22]. Table 1 shows the characteristics of the bearings. Figure 3 shows the shear force-displacement relationship as a function of axial load, as calculated by the theoretical model. It is



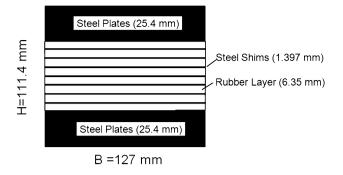


Fig. 2 Elastomeric bearing

possible to see that the displacements increase linearly with the lateral shear force. The axial load affects the stiffness that decreases with increasing axial load.

#### Lab test model

Shear force-horizontal displacement (F-v curves) from Nagarajaiah and Ferrell [22] have been here reproduced and compared first with the theoretical approach and then with numerical simulations. In particular, the F-v relationship has been calculated as a function of axial load for the specific type of bearing (named 302). The axial load was varied in the study: 20 kip (89 kN), 30 kip (133 kN) and 40 kip (188 kN), 50 kip (222 kN).

The results show that the force increases with the displacement but after a certain level of deformation, the force starts decreasing. Secondly, the shear force and horizontal displacement at which the maximum occurs decrease with increasing axial load. This non-linear behavior has been compared with the analytical results predicted by Forcellini and Kelly [12] in Fig. 4. Several considerations can be taken. First of all, the theory is able to catch the influence of the axial load. In particular, when the axial load increases, the stiffness decrease, as resulted from the tests. Secondly, in the first part of the curve, the theoretical approach can predict lab tests with good correspondence.

The comparison generally shows that analytical solutions have a satisfactory agreement with the test results for small values of force and displacements, at least when the relationship can be considered linear. After the peak value is reached, the theory loses its representativeness and is not able to describe the tested behavior because the structural parts such as hinges and beams start to register non-linear behaviors. In order to consider non-linear effects, Nagarajaiah and Ferrell [22] introduce a formulation to

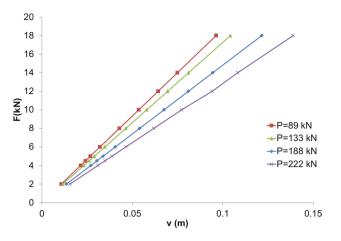


Fig. 3 Shear force-displacement curve following Forcellini and Kelly [12]

vary G as a function of the displacement. In particular, this variation was estimated empirically from the horizontal tangential stiffness, resulting from quasi-static tests. This paper, instead, aims at performing the tested behavior, by applying a highly non-linear software such as OpenSees (Open System for Earthquake Engineering Simulation) by Mazzoni et al. [20]. In this regard, the next paragraph details the finite 3D model applied in the study.

# Numerical simulations

This paragraph aims at reproducing lab test responses of the selected elastomeric bearings with a 3D numerical model. The finite element model (Fig. 5) was built with OpenSees [20], by the Pacific Earthquake Engineering Research (PEER) Center. This platform allows high level of advanced capabilities for modelling and analyzing nonlinear responses of systems using a wide range of material models, elements and solution algorithms. The paper applies the interface named OpenSeesPL (http://soilquake. net/openseespl/) that was developed at University of California, San Diego, by Lu et al. [18]. It consists of a framework originally calibrated for soil analyses. Here it has been modified by considering a layered bearing instead of a layered soil.

The adopted 3D (0.127 m  $\times$  0.127 m  $\times$  0.1114 m) FE mesh is composed of 19,272 brickUP linear isoparametric 8-node elements with 20,202 nodes (Fig. 5). The model base boundary is set at a depth of 0.1114 m and was considered with a fixed conditions. Lateral boundaries were set as a shear type, in order to simulate the typical shear deformation

 Table 1 Characteristics of the studied bearing

<i>A</i> [m <sup>2</sup> ]	G [MPa]	<i>K</i> [MPa]	<i>t</i> <sub>R</sub> [m]	S	P <sub>E</sub> [kN]	P <sub>S</sub> [kN]	$\lambda^2$
0.016	1.40	2000	0.0508	5	6000	26.72	0.00445

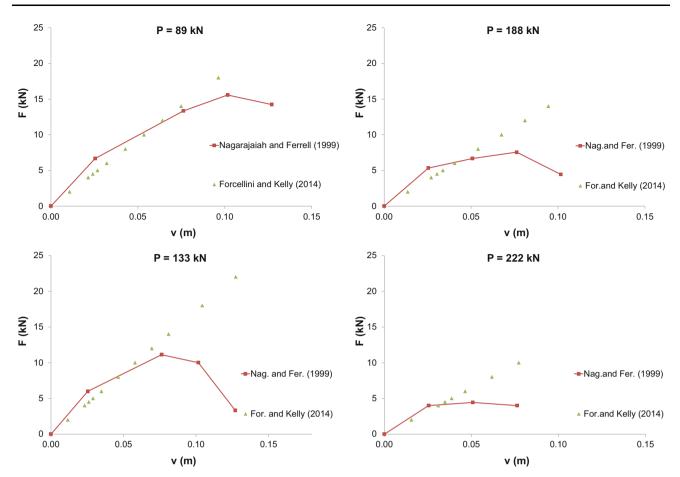


Fig. 4 Comparison between test results from Nagarajaiah and Ferrell [22] and theoretical results predicted by Forcellini and Kelly [12]

of elastomeric bearings. For all analyses, the Newmark transient integrator is used (with  $\gamma = 0.6$  and  $\beta = 0.3$ ). Stiffness and mass proportional damping is added with a 2% equivalent viscous damping at 1 and 6 Hz.

Based on previous studies [11], two materials have been considered, by applying the defined Pressure Independent Multiyield model. It consists of a nonlinear hysteretic material, using a Von Mises multi-surface kinematic plasticity approach together with an associate flow rule [23]. It allows controlling the magnitude of permanent shear strain accumulation [33]. Non-linear shear stressstrain backbone curve is represented by a hyperbolic relation, which is defined by low-strain shear modulus and ultimate shear strength constants.

The performed analysis aims at reproducing the test conditions under which the elastomeric bearings have been originally tested. In this regards, pushover analyses (displacement-based method) were performed. In particular, final displacements have been assigned at the top nodes. All the nodes at the top level were tied together in order to represent a rigid plate and thus they move at the same quantity as the final displacement. The corresponding stress and strain in the various layers have been calculated and the resultant forces deduced directly from the interface. Numerical simulations were performed in two steps. In the first, both materials (rubber and steel) have been assumed linear. In the second, steel has been still considered linear while rubber has been modelled with different non-linear assumptions detailed in 4.2.

#### Step 1 (linear model)

The first step consists in performing the model with linear assumptions in order to reproduce the theoretical results from Forcellini and Kelly [12]. Table 2 details the characteristics for the rubber and the steel shims (in terms of shear modulus, G and bulk modulus K). The values for the rubber have been taken from those proposed by Nagarajaiah and Ferrell [22]. Figure 6 shows the backbone (linear) curves considered for the rubber and steel, respectively. Figure 7 shows shear strain in correspondence with the rubber and the steel layer. It is possible to verify that the deformations are concentrated in the rubber, as expected. The entity of such deformation corresponds to the ratio between the applied longitudinal displacement and the height of the rubber ( $t_R$  in Table 1). This means that the model is able to represent the imposed deformations.

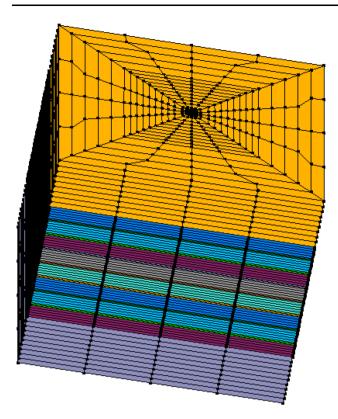


Fig. 5 3D FEM model

Longitudinal displacements are shown in Fig. 7 and they show a shear-type behavior, as expected.

Figure 8 shows that elastic assumptions allow obtaining results that are close to the theoretical approach. Therefore, theoretical formulation in terms of large deformation is able to represent correctly the behavior (such as shear deformations) when linear assumptions are made. On the other side, these numerical simulations (with linear assumptions) cannot represent the non-linear behavior that resulted from the tests (Table 2).

#### Step 2 (non-linear model)

In the second step, the material named Pressure Independent Multivield [20] has been calibrated in order to represent the non-linear behavior described during lab tests. In particular, steel material has been maintained with the same characteristics of the first step. Rubber has been modelled as a non-linear hysteretic material with a Von Mises multi-surface kinematic plasticity model. In this regard, focus is on reproduction of the hysteretic elastoplastic shear response (including permanent deformations). In this material, plasticity is exhibited only in the deviatoric response. The volumetric stress-strain stress-strain response is linear-elastic and independent of the deviatoric response. Plasticity is formulated based on the multi-surface (nested surfaces) concept, with an associative flow rule. The non-linear shear stress-strain backbone curve is represented by the hyperbolic relation, defined by two material constants (low-strain shear modulus and ultimate shear strength).

The parameters adopted for the rubber were calibrated in order to take into account the decreasing of the shear forces with the increasing of the displacements. As it was built up, the material is able to catch the non-linear increase of force but not the degradation after the peak value. In particular, the peak value has been defined as the tension after which cavitation is supposed to start. This level of stress was set equal to 3G and maintained constant. Table 3 shows the values that have been calibrated. In particular, C is the shear stress peak value that is reached in correspondence with the shear strain named p. n is the number of yield surfaces considered. Figure 9 shows the theoretical Von Mises multi-surface in principal stress space and the shear stress-strain backbone curve adopted for the rubber.

Figure 10 shows the shear force-displacement curves obtained from the numerical simulations. First of all, it is

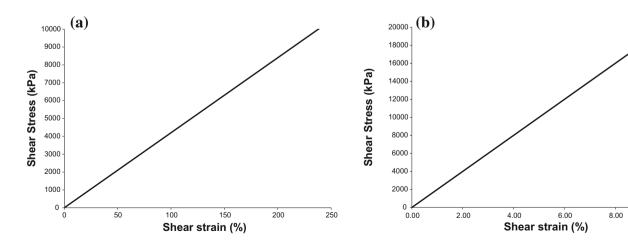


Fig. 6 Backbone curve—step 1 [rubber (a), steel (b)]

10.00

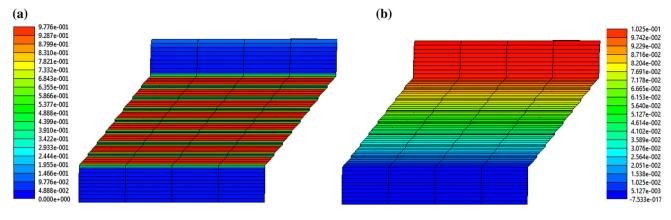


Fig. 7 Output. a shear strain (last step) and b displacements (last step, scale 1:1)

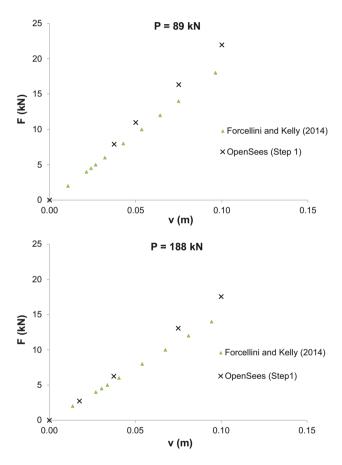


Fig. 8 Shear force-displacement curve (linear model)

 Table 2 Characteristics of the material (step 1)

G <sub>rub</sub> [MPa]	<i>K</i> <sub>rub</sub> [MPa]	G <sub>steel</sub> [MPa]	K <sub>steel</sub> [MPa]
1.40	2000	$8.04 \times 10^4$	$1.74 \times 10^4$

possible to see that the curves decrease with increasing axial load, as shown by Nagarajaiah and Ferrell [22]. Secondly, numerical simulations are not able to perform

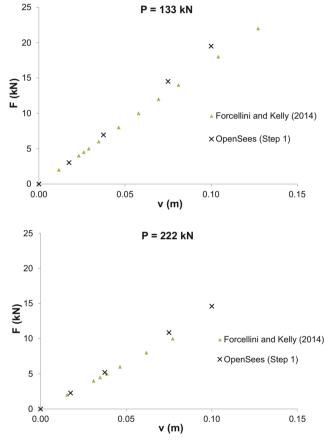


 Table 3 Characteristics of the material (step 2)

G <sub>rub</sub> [MPa]	<i>K</i> <sub>rub</sub> [MPa]	C [MPa]	p [%]	n
1.40	2000	4.20	0.01	20

the degradation after the peak, even if OpenSees takes into account non-linear effects such as large displacements and *p*-delta effects.



-P=89 kN

-P=133 kN

0.15

→P=188 kN →P=222 kN

0.10

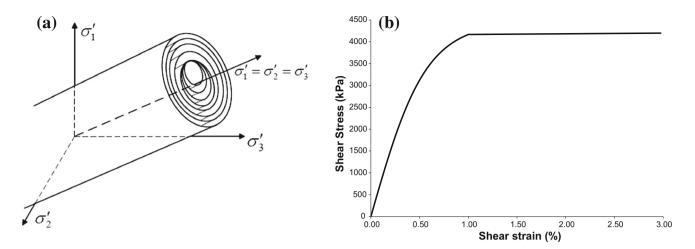


Fig. 9 Von Mises multi-surface in principal stress space (a) and shear stress-strain backbone curve (b) for the rubber (step 2)

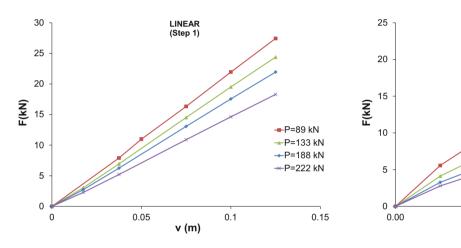


Fig. 10 Shear force-displacement curve (OpenSees)

# Discussion

Figure 11 shows the comparison between the shear force– displacement curves obtained by Nagarajaiah and Ferrell [22], the theoretical approach from Forcellini and Kelly [12] and OpenSees simulations with the two steps (linear and non-linear) at various axial loads: 20 kip (89 kN), 30 kip (133 kN) and 40 kip (188 kN), 50 kip (222 kN). In particular, numerical results show that the material is able to catch the non-linear increase. Therefore, the parameters with the numerical simulations built up, are shown to be correctly assumed. However, the performed behavior is not able to represent what happens after the peak value. OpenSees is able to model many non-linear effects such as large displacements and p-delta effects, but it cannot properly represent the degradation after the peak. This behavior is presumably due to some phenomenon (such as cavitation) that is not described by either the theoretical approach of large displacements or the OpenSees nonlinearities.

### **Further development**

0.05

The stress level registered during the tests is close to the level at which cavitation is supposed to occur. In particular, cavitation consists of microvoid formations and causes the loss of experimentally observed stiffness when rubber is subjected to hydrostatic tension or dilatant stress [8]. The authors show that cavitation damage can be associated with a significant variation of the bulk modulus K. In this regard, the theoretical approach has to be developed by introducing a more sophisticated definition that can take into account this variation (actually, K has been considered constant). In addition, Dorfmann and Burtscher [8] have shown that cavitation depends on the dilatational part of the energy density functional. Dorfmann et al. [9] proposed a formulation based on a pseudo-elastic model that can be applied to finite element simulations. In this regard, Kumar et al. [14] have investigated cavitation with numerical investigations by implementing the proposed mathematical model inside OpenSees as two new elements. In particular,

NON LINEAR

(Step 2)

v (m)

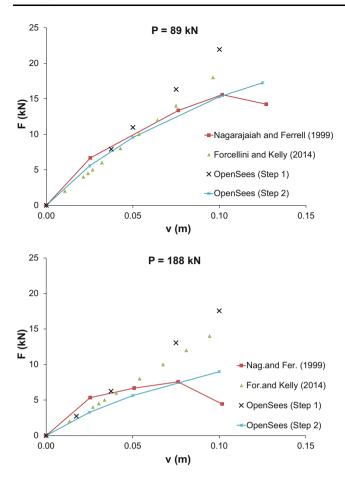


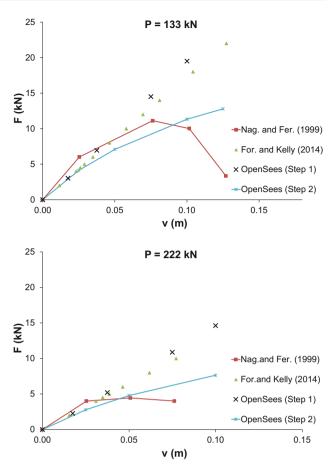
Fig. 11 Shear force-displacement curve

the next step will be to perform 3D numerical simulations by applying a material that will allow considering cavitation. This model will be compared with the elements already implemented by Kumar et al. [14].

### Conclusions

This paper performs a comparison between the theoretical approach proposed by Forcellini and Kelly [12], experimental results from Nagarajaiah and Ferrell [22] and numerical simulations of an elastomeric bearing. The shear force–horizontal displacement, F-v curves have been here reproduced as a function of axial load that has been varied: 20 kip (89 kN), 30 kip (133 kN) and 40 kip (188 kN), 50 kip (222 kN). Several steps have been followed.

The theory based on large displacement assumptions has been compared with the lab tests. The comparison shows a good agreement in the first part of the curves where levels of shear load and displacements are small (and linear assumptions can be considered). In particular, it was shown that the effects of the axial load have been performed properly by both the analytical solution and numerical simulations.



Then, a 3D finite element model has been modelled by applying the interface OpenSeesPL. In particular, a selected device has been modelled. Two subsequent assumptions for the material have been considered. First, both materials (rubber and steel) have been considered linear. Then, rubber material has been modelled with a highly hysteretic material, with a Von Mises multi-surface kinematic plasticity model (named Pressure Independent Multiyield).

The comparison between the various results confirms that numerical results have satisfactory agreement with the test results and analytical solution. In this regard, the study does not take into consideration the cavitation phenomenon that can be the most credited cause for the degradation of shear stress. Future work will improve the model in order to take into account cavitation, aiming to be more representative and catch the decreasing part of the curve.

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